

# Update on NIST's Investigation of the Partial Collapse of Champlain Towers South in Surfside, Florida

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History Project*



**STRUCTURES CONGRESS 2025** | [structurescongress.org](https://structurescongress.org)

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Genesis of the  
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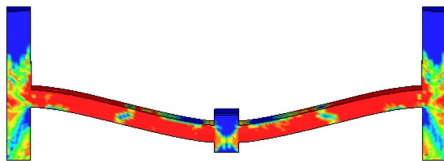
07

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The Partial Collapse,  
Genesis of the  
Investigation

# Disaster Resilience Work across NIST (and scales)



Infrastructure  
Materials  
Science

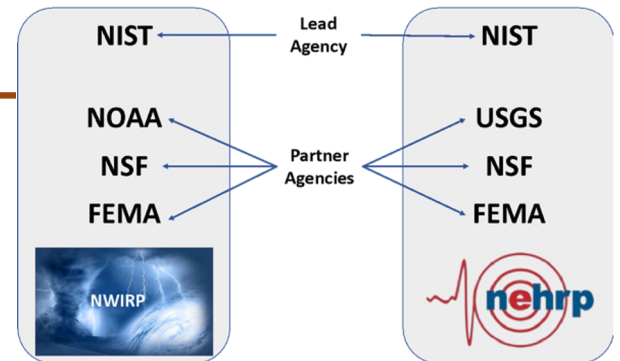
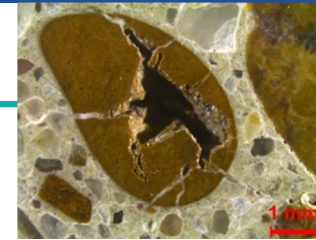
Structural  
Eng. &  
Earthquake  
Eng.

Community  
Resilience



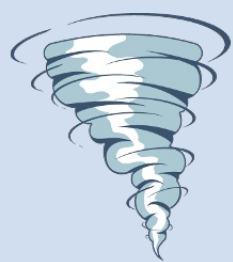

Field Studies  
NCST  
Investigations

Interagency  
Leadership

Extramural  
Programs



# Long History of Disaster Studies at NIST

Earthquakes	Hurricanes	Construction & Building	Tornadoes	Fires
<p>San Fernando, CA (1971)                      Mexico City, Mexico (1985)                      Loma Prieta, CA (1989)                      Northridge, CA (1994)                      Kobe, Japan (1995)                      Kocaeli, Turkey (1999)                      Maule, Chile (2010)                      Christchurch, NZ (2011)</p> <p><b>*Ongoing Studies</b>  <b>NCST Investigations</b></p> 	<p>Camille, MS/LA (1969)                      Alicia, Galveston, TX (1983)                      Hugo, SC (1989)                      Andrew, FL (1992)                      Fran, NC (1996)                      Mitch and Georges, LAC (1998)                      Katrina and Rita (2005)  <b>*Matthew &amp; Florence, NC (2016 &amp; 2018)</b>                      Harvey, TX (2017)  <b>*Maria, PR (2017)</b></p>	<p>Skyline Plaza Apartments, Bailey's Crossroads, VA (1973)                      Willow Island Cooling Tower, WV (1978)                      Kansas City Hyatt Regency, Kansas City, MO (1981)                      Riley Road Interchange, East Chicago, IN (1982)                      Harbor Cay Condominium, Cocoa Beach, FL (1981)                      L'Ambiance Plaza, Hartford, CT (1987)                      Ashland Oil Tank Collapse, Floreffe, PA (1988)                      U.S. Embassy, Moscow, USSR (1987)                      Murrah Federal Building, Oklahoma City, OK (1995)  <b>World Trade Center Disaster, New York, NY (2001)</b>                      Dallas Cowboys Indoor Practice Facility, May 2009  <b>*Champlain Towers South, Surfside, FL (2021)</b></p> 	<p>Jarrell, TX (1997)                      Spencer, SD (1998)                      Oklahoma City, OK (1999)  <b>Joplin, MO (2011)</b>                      Moore OK (2013)</p> 	 <p>DuPont Plaza Hotel, San Juan, PR (1986)                      First Interstate Bank Building, Los Angeles, CA (1988)                      Loma Prieta Earthquake, CA (1989)                      Hillhaven Nursing Home (1989)                      Pulaski Building, Washington, DC (1990)                      Happyland Social Club, Bronx, NY (1990)                      Oakland Hills, CA (1991)                      Watts St, New York City (1994)                      Northridge Earthquake, CA (1994)                      Kobe, Japan (1995)                      Vandalia St, New York City (1998)                      Cherry Road, Washington, DC (1999)                      Keokuk, IA (1999)                      Houston, TX (2000)                      Phoenix, AZ (2001)                      Cook County Administration Building Fire (2003)  <b>The Station Nightclub, RI (2003)</b>                      Charleston, SC, Sofa Super Store Fire (2007)                      Witch Creek &amp; Guejito, CA, WUI Fire (2007)                      Amarillo, TX, WUI Fire (2011)                      San Francisco, CA (2012)                      Gatlinburg, TN WUI (2016)                      Fuse-47, MD (2017)</p>

# National Construction Safety Team (NCST) Act

## NCST Act\* (Oct. 1, 2002)

The National Construction Safety Team Act was enacted to provide for the establishment of investigative teams (“Teams”) **to assess building performance and emergency response and evacuation procedures in the wake of any building failure that has resulted in substantial loss of life or that posed significant potential of substantial loss of life.**

## Unique to NCST

- Subpoena authority
- NIST investigator credentials
- **Federal advisory committee (up to 12 appointed members)**
- Follow through on recommendations and report(s) to Congress

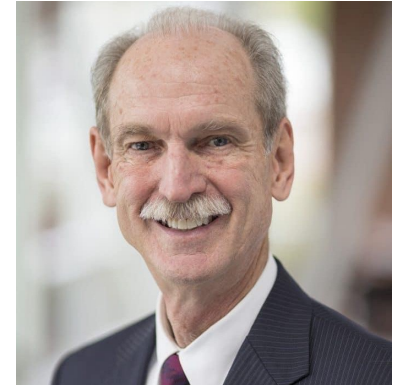
\*National Construction Safety Team (NCST) Act (Public Law 107-231, codified at 15 U.S.C. 7301 et seq.) and the Implementing Regulations (15 C.F.R. Part 270).



José Izquierdo-Encarnación



Kimberly Shoaf



Donald Dusenberry



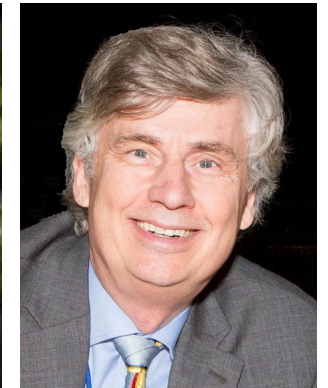
Lori Peek



Kurtis Gurley



Aspasia Zerva



John Oстераas



## Press Conference on June 30, 2021 launches NCST investigation

James K. Olthoff

Performing the non-exclusive functions and duties of NIST Director

Source: NIST

# NIST Coordinates Evidence Handling, and Establishes Evidence Tagging Protocols, with First Responders & Incident Command



1. INCIDENT NAME:  
Champlain Towers  
Building Collapse

2. DATE PREPARED:  
7/01/2021

3. TIME PREPARED:  
2000 hrs

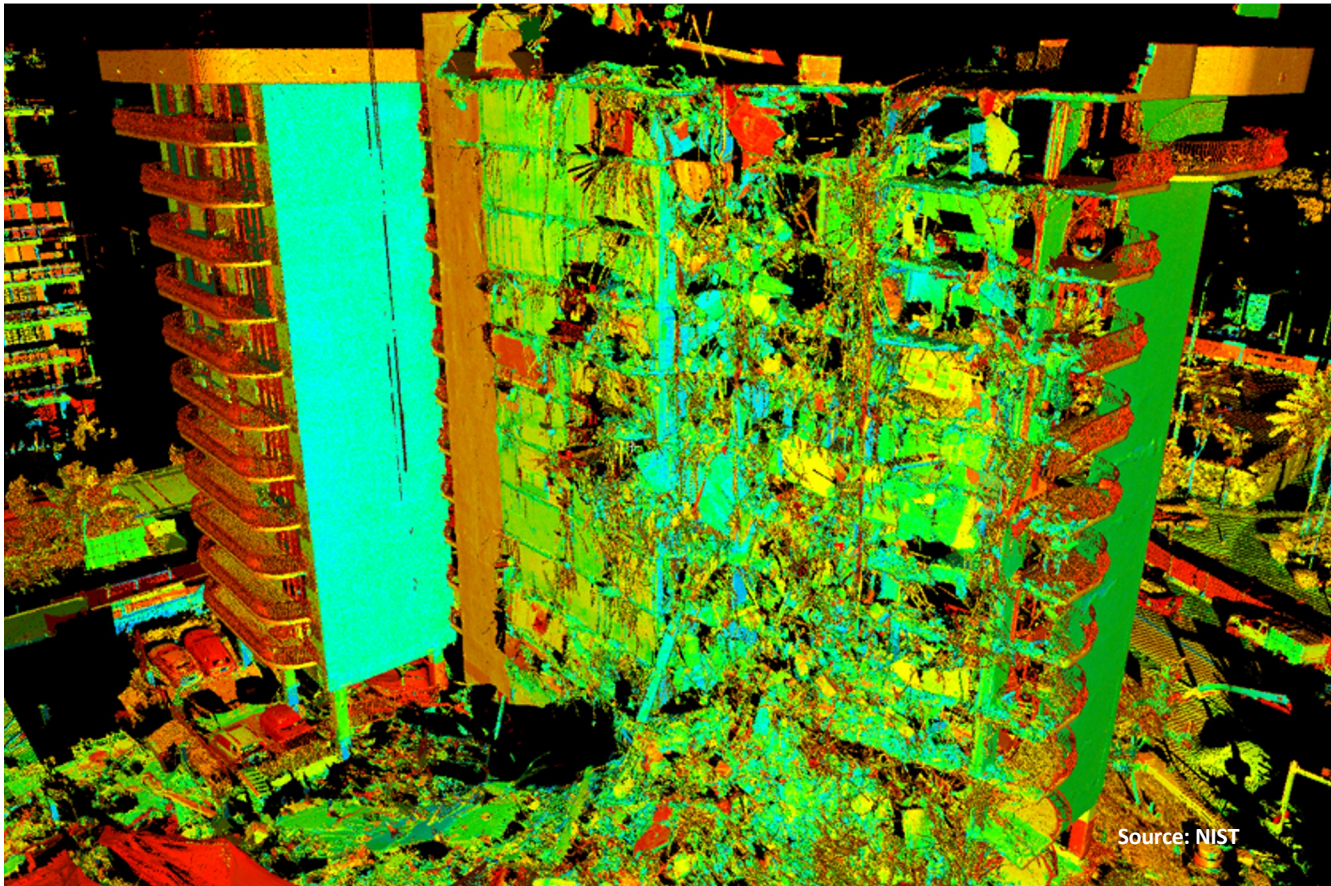
## EVIDENTIARY RUBBLE HANDLING PROCEDURE

Source: Miami-Dade County





# Experts from FEMA, NSF NHERI RAPID, USGS, and USACE Support On-Site Remote Sensing Activities



# Experts from NSF NHERI RAPID, FSU, Miami-Dade Fire Rescue, and VA Beach Fire Dept. Support Remote Sensing Activities



## NIST Conducts a Subsurface Investigation with Experts from USACE and Georgia Tech (via NIST Disaster Resilience Research Grant)



Source: NIST

Source: NIST

# NIST Conducts Wave Attenuation Tests with Experts from NSF NHERI@UTexas, Utah State University, and Georgia Tech (DRRG)

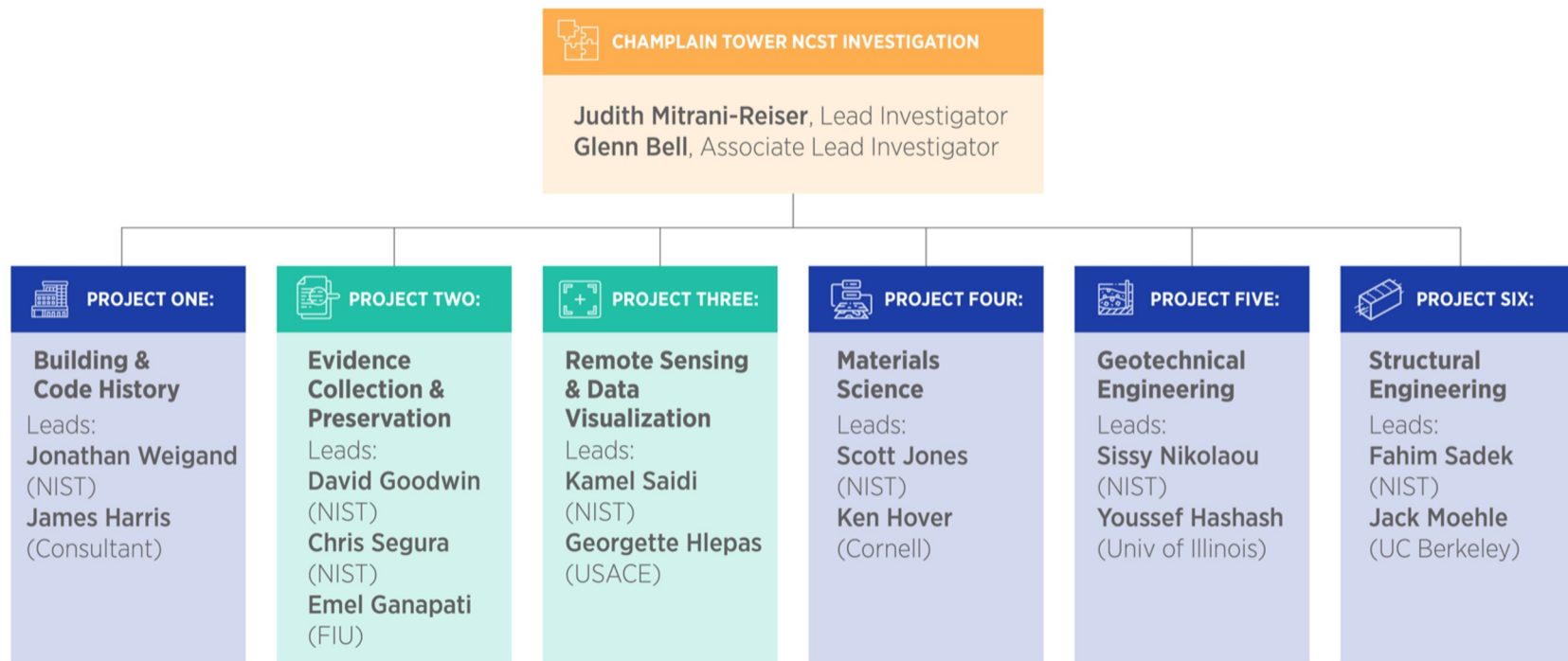


## Final Organization and Storage of CTS NCST Evidence



# Organization Structure of the Champlain Towers South NCST Investigation

## Champlain Towers South NCST Investigation Leaders



 COLLAPSE EVIDENCE ANALYSIS

 PROGRESSIVE COLLAPSE ANALYSIS

**Collaborate  
Coordinate  
Cooperate**

**NIST Engineering Laboratory (EL)**

Structures Group (MSSD)  
Infrastructure Materials Group (MSSD)  
Earthquake Engineering Group (MSSD)  
Community Resilience Group (MSSD)  
Disaster Statutory Programs (MSSD)  
Intelligent Systems & Fire Research Divisions  
EL's Data, Security, Technology Group  
EL's Applied Economics Office

MSSD = Materials and Structural Systems Division

**NIST**

Physical Measurement Laboratory  
Materials Measurement Laboratory  
Public Affairs Office  
Office of Chief Counsel  
Program Coordination Office  
Management and Organization Office  
Acquisition & Agreements Mgmt. Office  
ITL's Statistical Engineering Division

ITL = Information Technology Laboratory

**Federal**

Federal Emergency Mgmt. Agency  
U.S. Army Corps of Engineers  
U.S. Geological Survey  
National Science Foundation  
Federal Bureau of Investigation  
Department of Defense  
NOAA's National Weather Service  
Bureau of Reclamation

NOAA = National Oceanic and Atmospheric Administration

**Local and State**

Miami-Dade County Mayor's Office,  
Fire, Police, and Building Departments  
Town of Surfside  
City of Miami Beach  
Florida Division of Emergency Mgmt.  
Florida DOT and State Attorney's Office  
Virginia Beach Fire Department  
USAR Task Forces

DOT = Department of Transportation

USAR= Urban Search & Rescue

## Disclaimers

### **IMPORTANT: ALL DATA ARE PRELIMINARY**

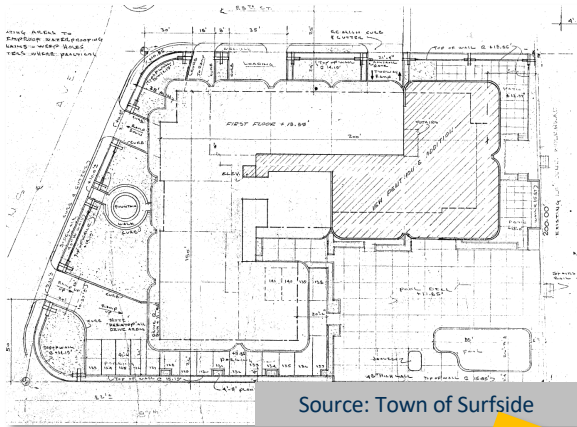
- These presentations describe preliminary data gathered to date as well as preliminary analyses of these data. Data and analyses are subject to change.
- Once all data are finalized and analyzed, they will inform a broader understanding of the likely technical cause or causes of the collapse – and NIST’s findings and recommendations.
- These presentations do not constitute NIST findings or recommendations.
- All survey and interview data collection included a consent process that specifies the allowable uses of data and protections of respondents.
- Copyrighted content (such as photographs) appearing in these presentations is used with permission; reproduction, redistribution or reuse may require copyright holder permission, including for content with anonymous attribution/credit.
- Every reasonable effort has been made to identify copyright holders for content (such as photographs) appearing in these presentations.



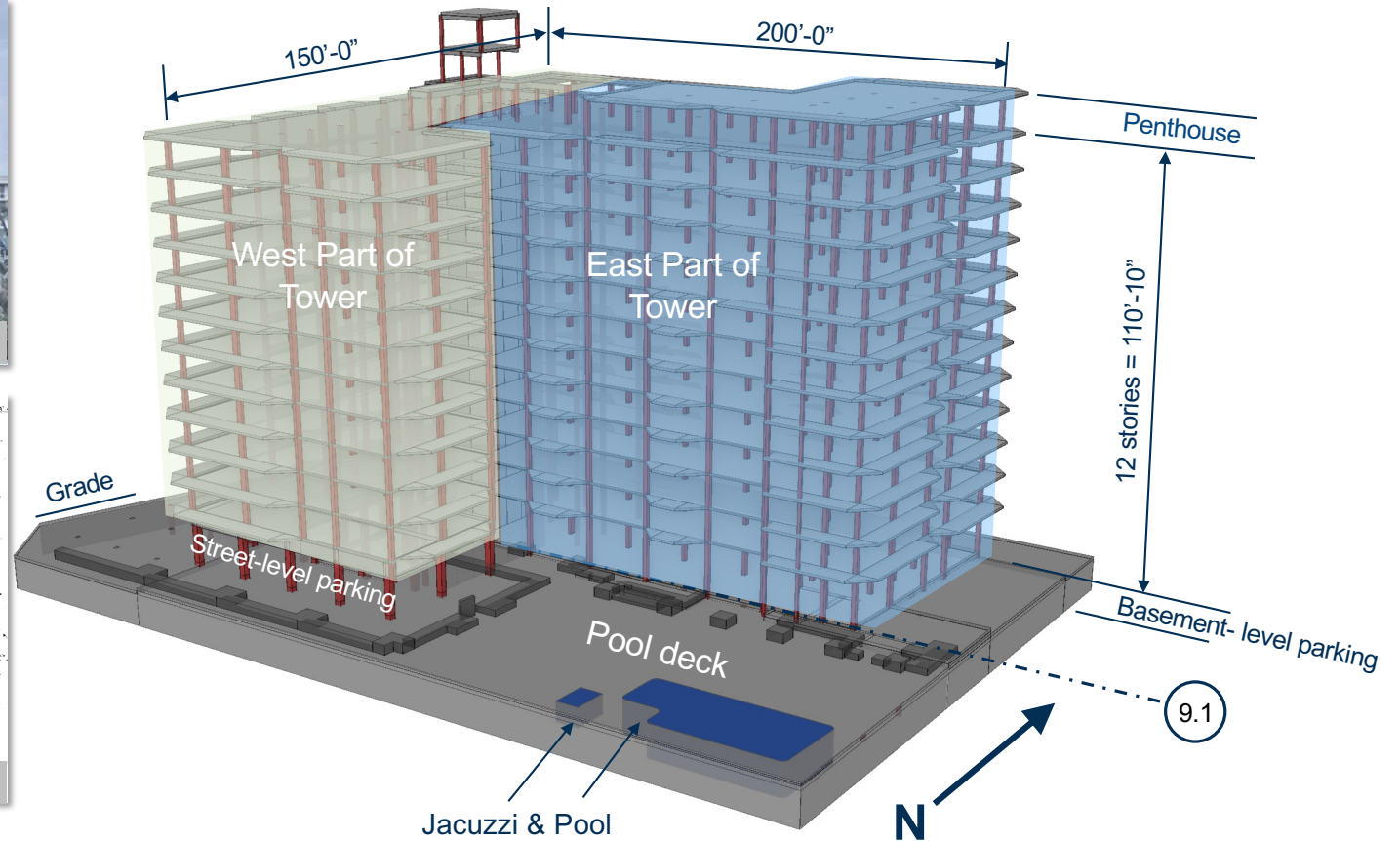
02

Description of Building,  
Investigative Approach

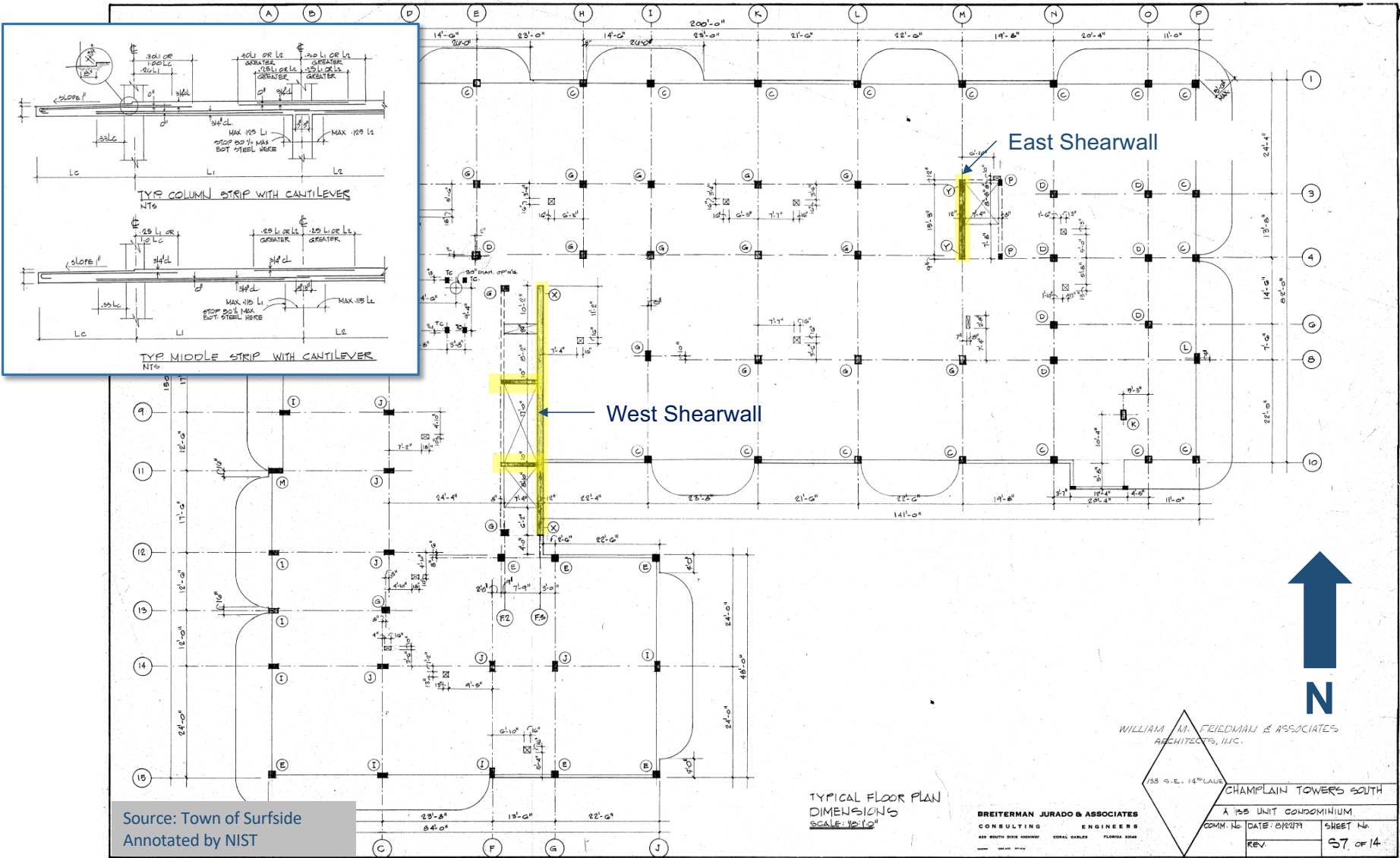
# Description of the Building



direction of view



Source: NIST



Source: Town of Surfside  
Annotated by NIST

TYPICAL FLOOR PLAN  
DIMENSIONS  
SCALE: 1/8" = 1'-0"

BREITERMAN JURADO & ASSOCIATES  
CONSULTING ENGINEERS  
430 SOUTH BAY HIGHWAY CORAL GABLES FLORIDA 33134

WILLIAM M. FRIEDMAN & ASSOCIATES  
ARCHITECTS, INC.

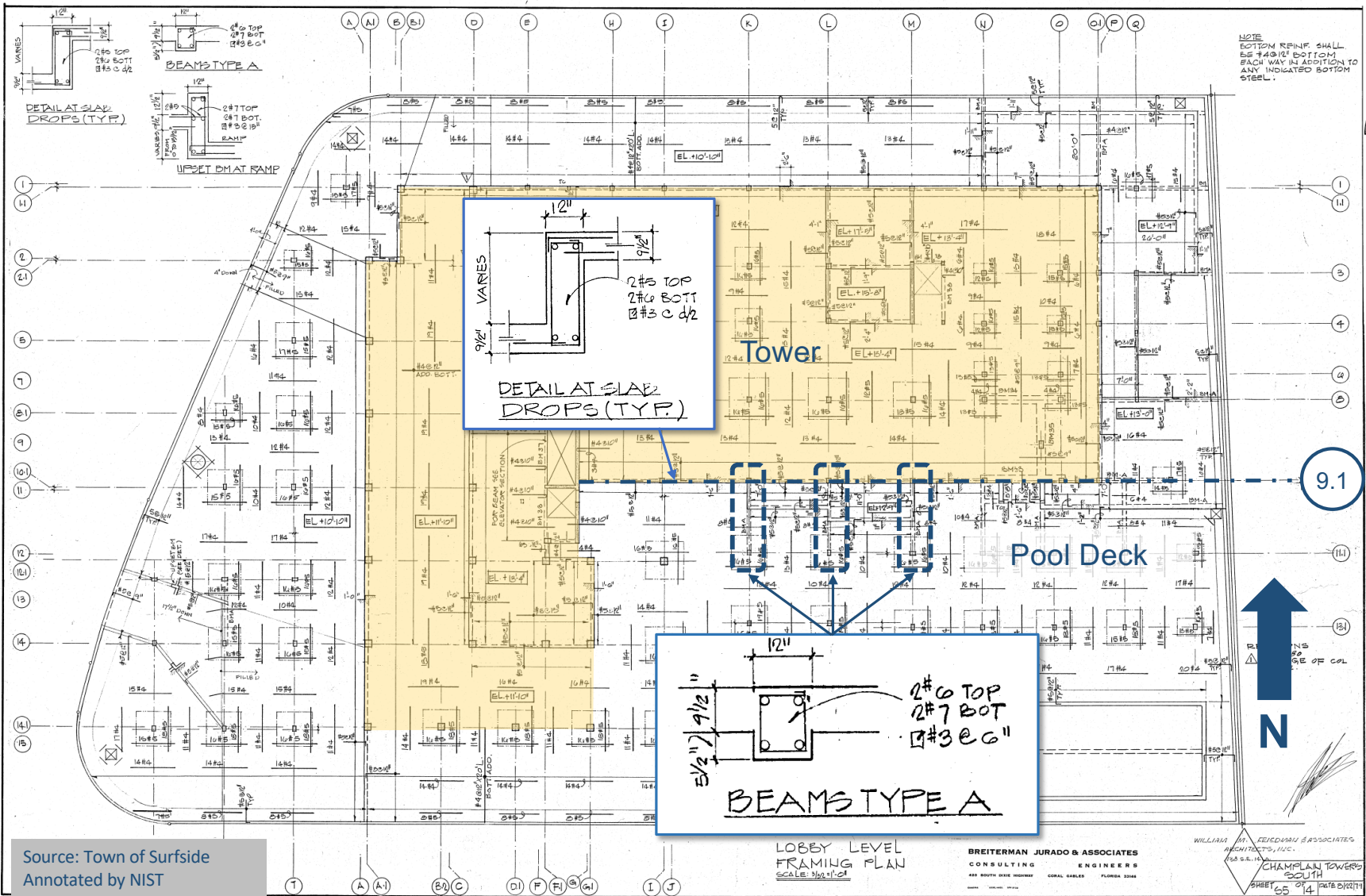
133 S.E. 14<sup>TH</sup> LANE

CHAMPLAIN TOWERS SOUTH

A 155 UNIT CONDOMINIUM

COMM. NO. DATE: 01/2017 SHEET No.

REV. S7 of 14



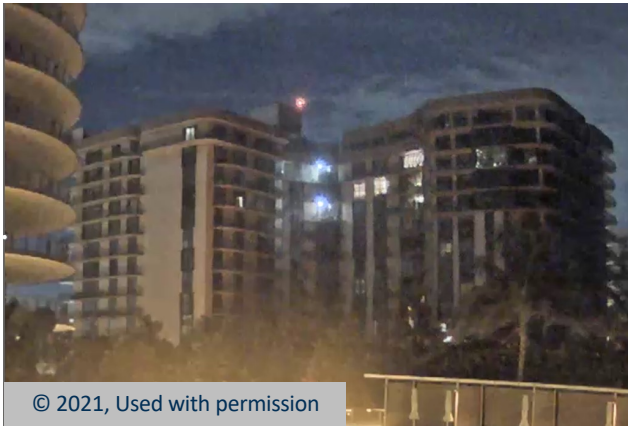
Source: Town of Surfside  
Annotated by NIST

LOBBY LEVEL  
FRAMING PLAN  
SCALE: 3/8" = 1'-0"

BREITERMAN JURADO & ASSOCIATES  
CONSULTING ENGINEERS  
430 SOUTH STATE HIGHWAY CORAL GABLES FLORIDA 33134

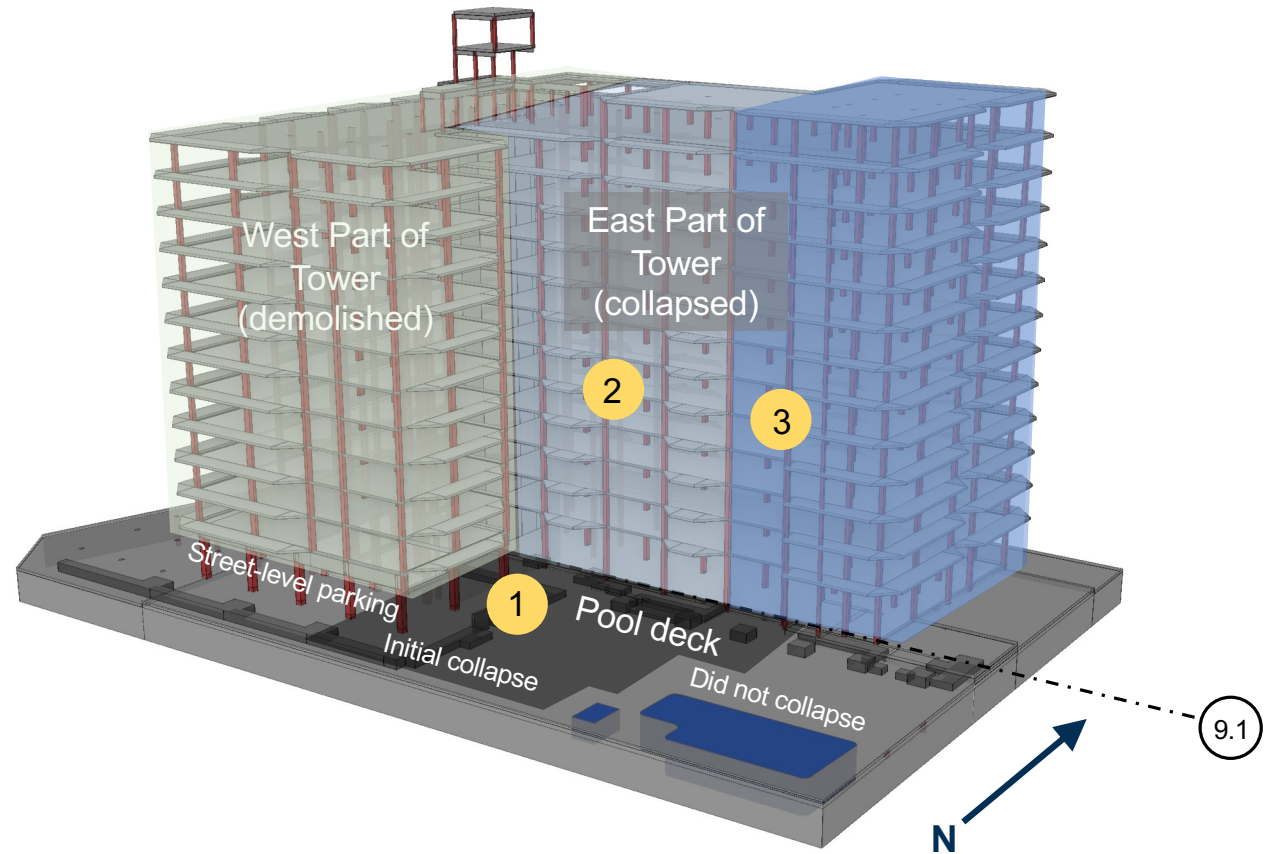
WILLIAM H. FEUERMAN & ASSOCIATES  
ARCHITECTS, P.C.  
550 S.W. 11th  
CHAMPLAN TOWERS  
SOUTH  
SHEET 25 OF 44 DATE 02/27/11

# The Partial Collapse



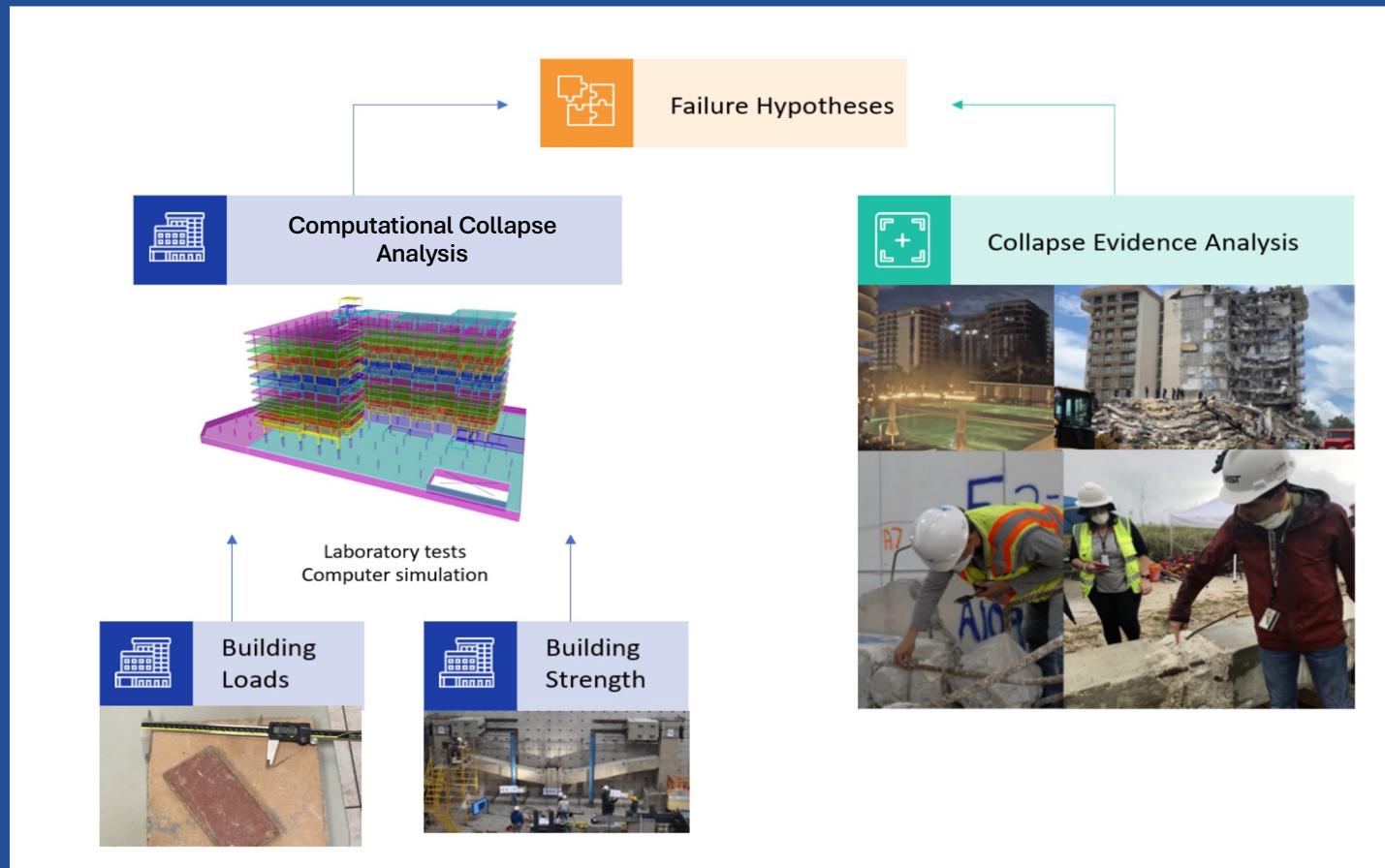
© 2021, Used with permission

South Face Video Footage

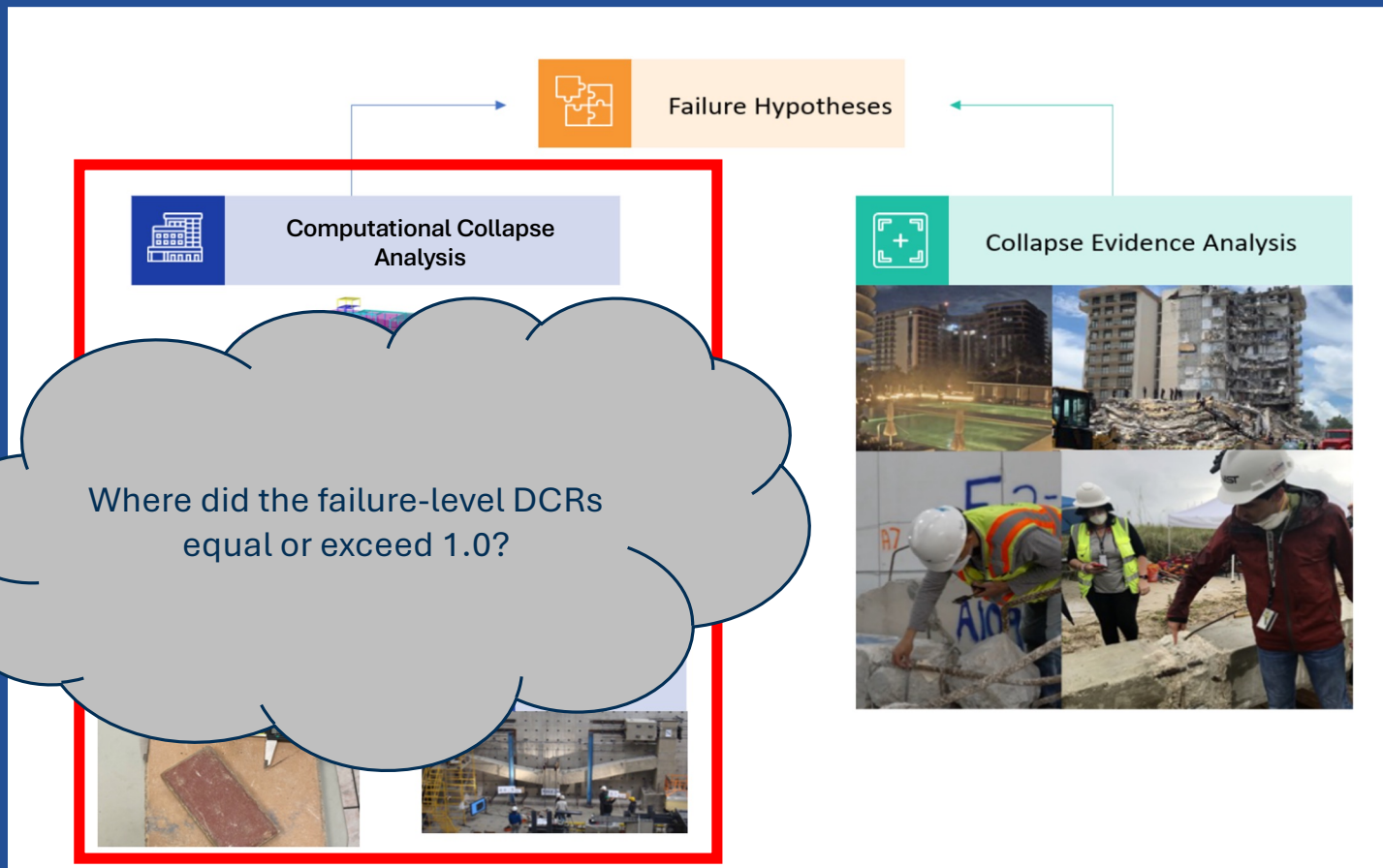


Source: NIST

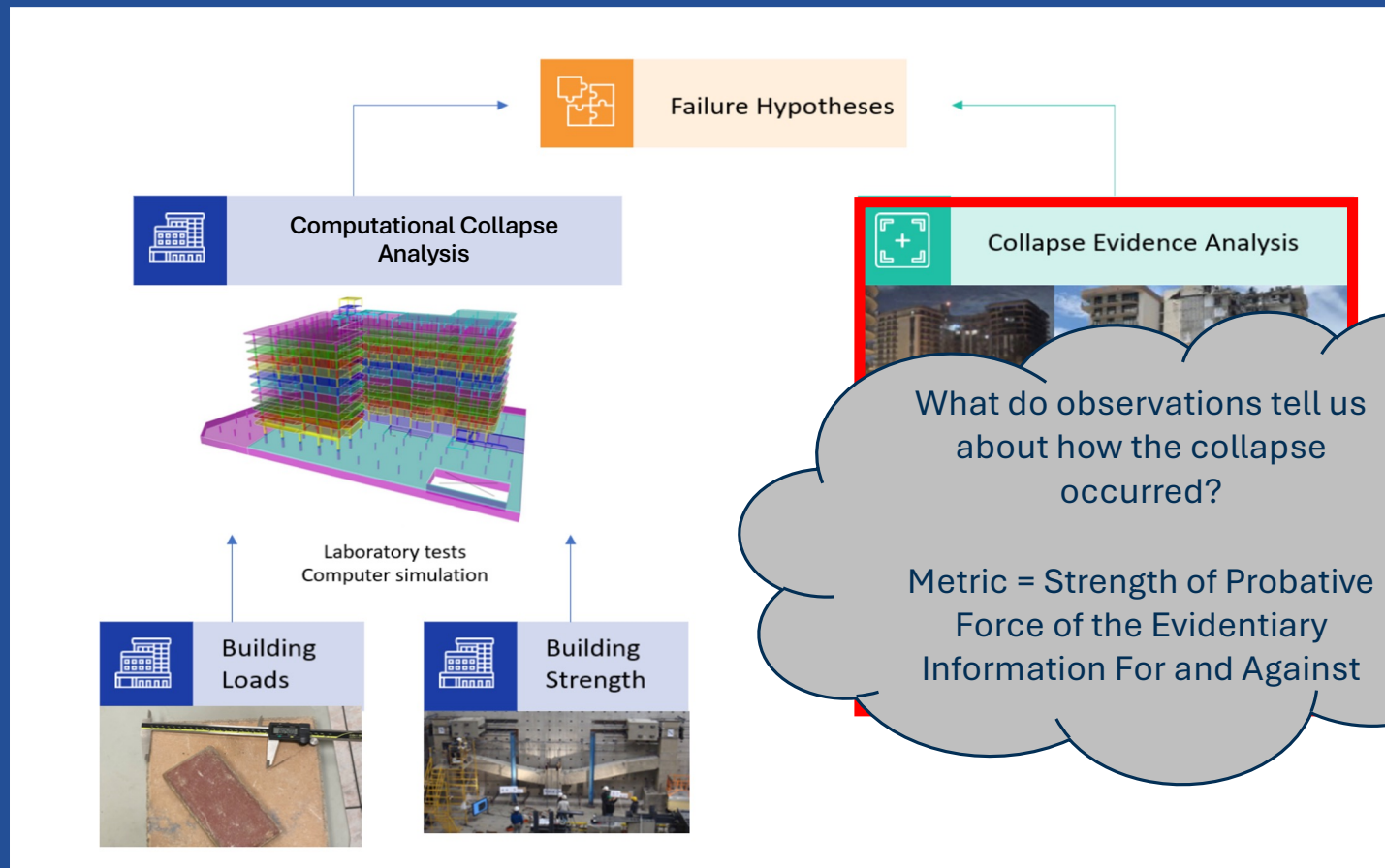
# Investigative Approach



# Investigative Approach

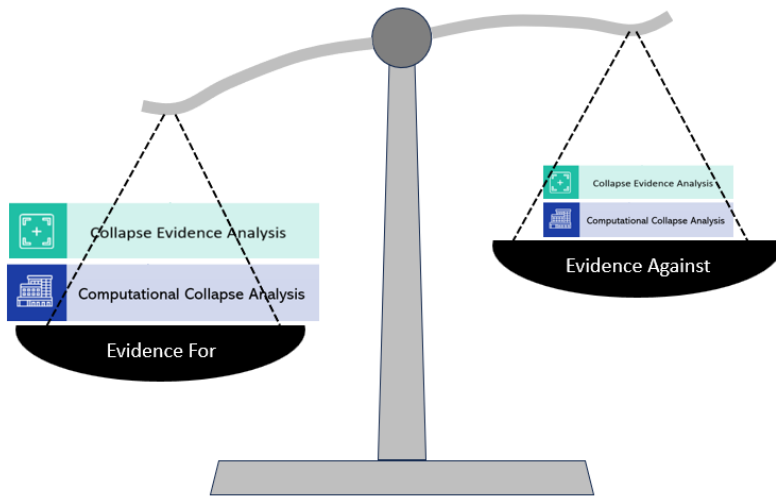


# Investigative Approach





# Weighing Evidence > Relative Likelihoods of Failure Hypotheses



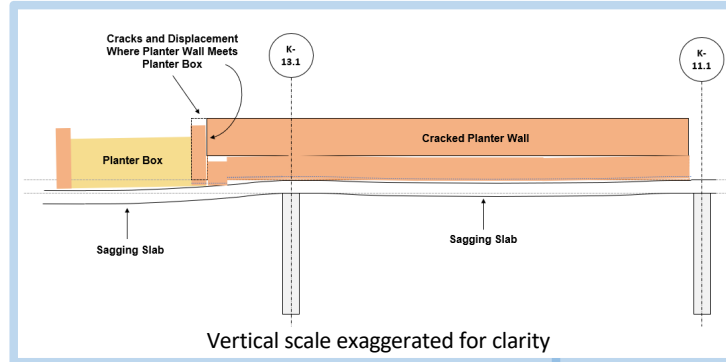
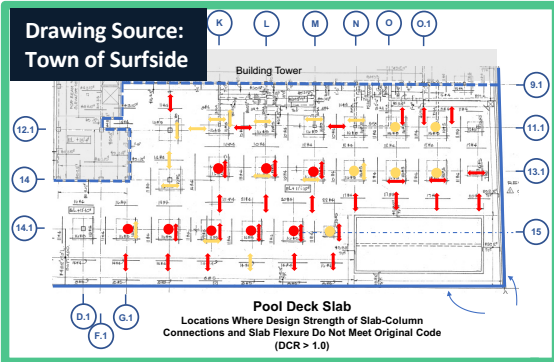
Most Likely Scenario or  
Scenarios

Everything Else  
(so unlikely that it's not mentioned  
above)

03

## Social Sciences, Collapse Timeline

# Timeline of the Site/Building History and Collapse



Site History

1900-1979

CTS Design & Construction

1979-1981

CTS Building History

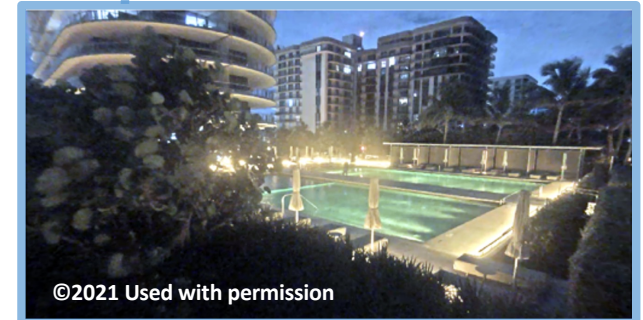
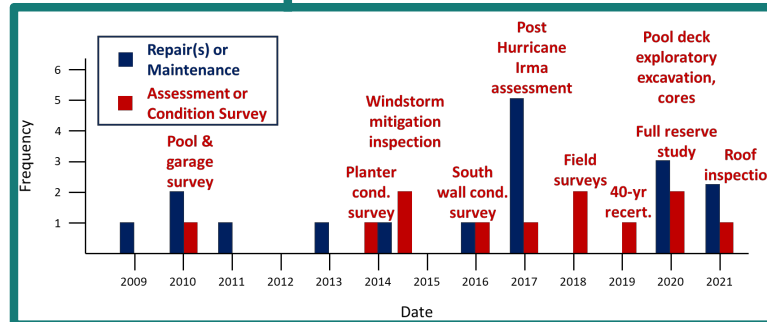
1981-2021

Champlain Towers South Collapse

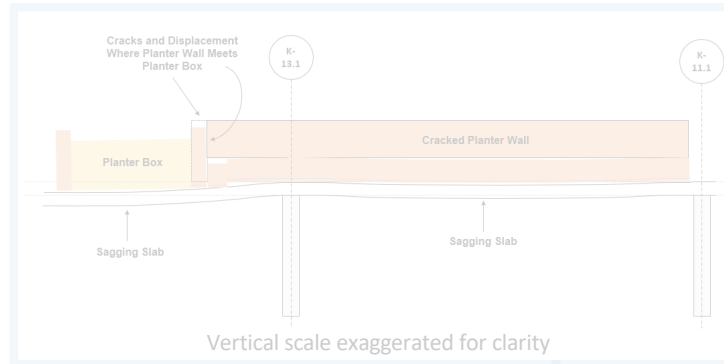
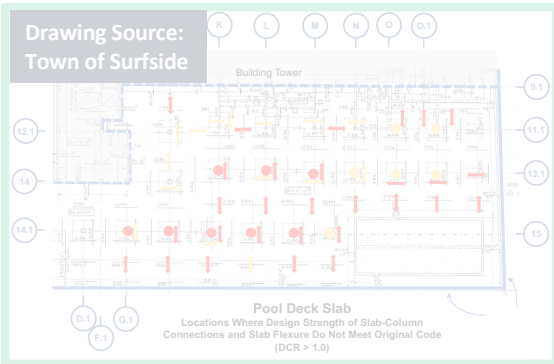
Weeks/Days Prior to Collapse

Hours/Minutes Prior to Collapse

Initiation & Progression



# Timeline of the Site/Building History and Collapse



Site History

1900-1979

CTS Design & Construction

1979-1981

CTS Building History

1981-2021

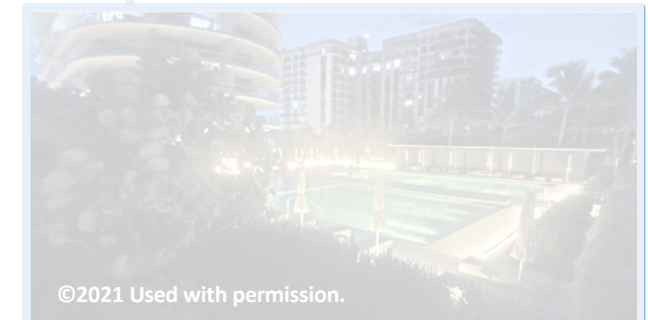
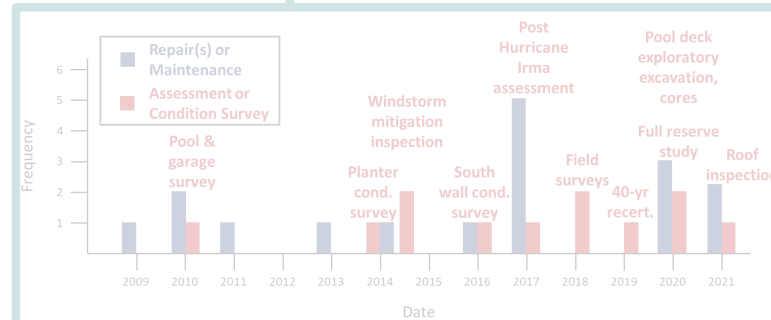
Champlain Towers South Collapse

**Visual Evidence Presented in Slides Spans 4 min 14 s**

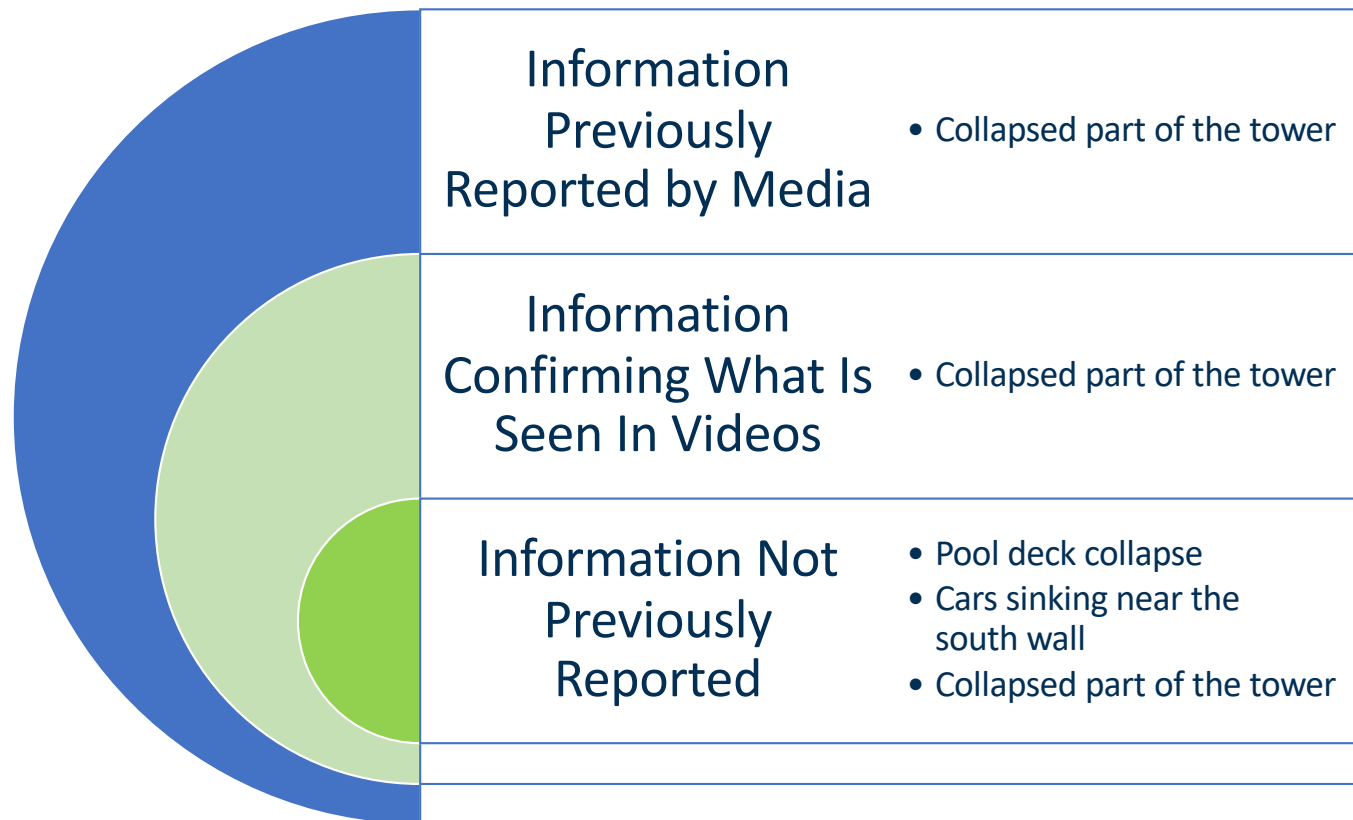
Weeks/Days Prior to Collapse

Hours/Minutes Prior to Collapse

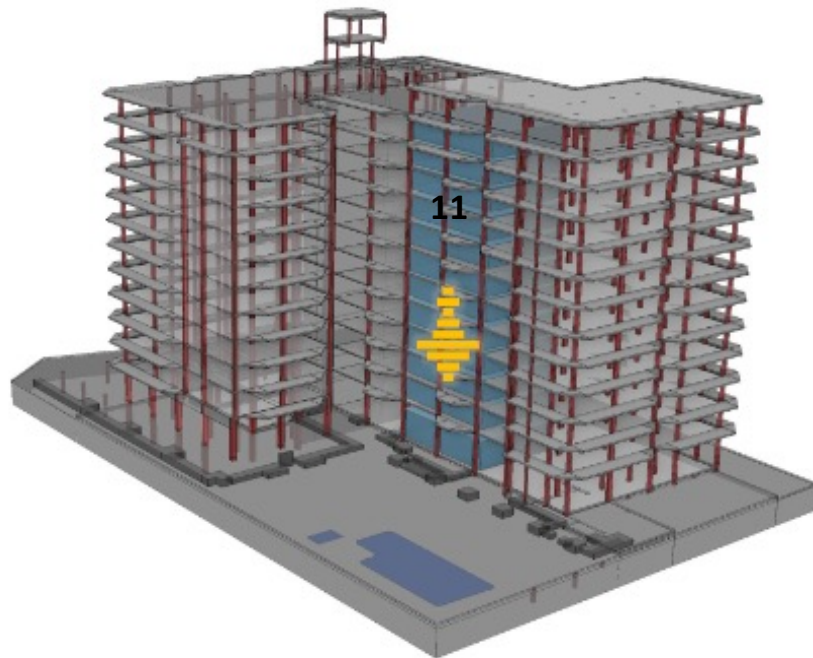
Initiation & Progression



# Timeline of Collapse Initiation and Progression is Informed by Interviews



## Timeline of Collapse Initiation and Progression is Informed by Interviews



Source: NIST

Noises in the 11 stack units from above the 1st floor before 1 am

Like “knocking” or a “hammer” or “table” & “chairs” being moved above

Noises getting “louder and louder” and “more intense” closer to the time of the pool deck collapse

# Timeline of Collapse Initiation and Progression is Informed by Interviews



Source: NIST



Direction of cars sinking

Car alarms "going off"

Cars sinking into the ground near the south wall from the East towards the West

Sinking does not "go all the way" to Collins Avenue

# Timeline of Collapse Initiation and Progression



01:18:18

01:21:50

~01:22

Visual Evidence Presented in Slides Spans 4 min 14 s

01:21:39

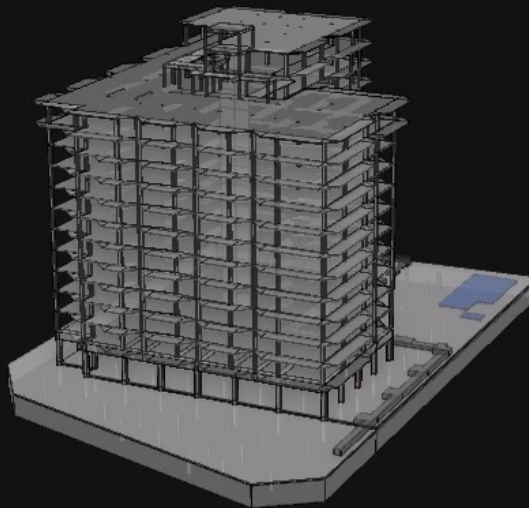
01:22:14

01:22:17





# Footage of Collapse Initiation and Progression



Source: NIST

## Footage of Collapse Initiation and Progression

### **CONTENT WARNING:**

The following slides contain images, video footage, and other content that some may find disturbing.

Participants desiring to leave the meeting may do so now.

# Timeline of Collapse Initiation and Progression: Ramp Video



01:18:18

Visual Evidence Presented in Slides Spans 4 min 14 s

01:21:50

~01:22

01:21:39

01:22:14

01:22:17



1:18:18 AM 6/24/2021

6' 5"

DO NOT TOUCH  
IF YOU SEE THE  
REAR VIEW  
302-334-2100

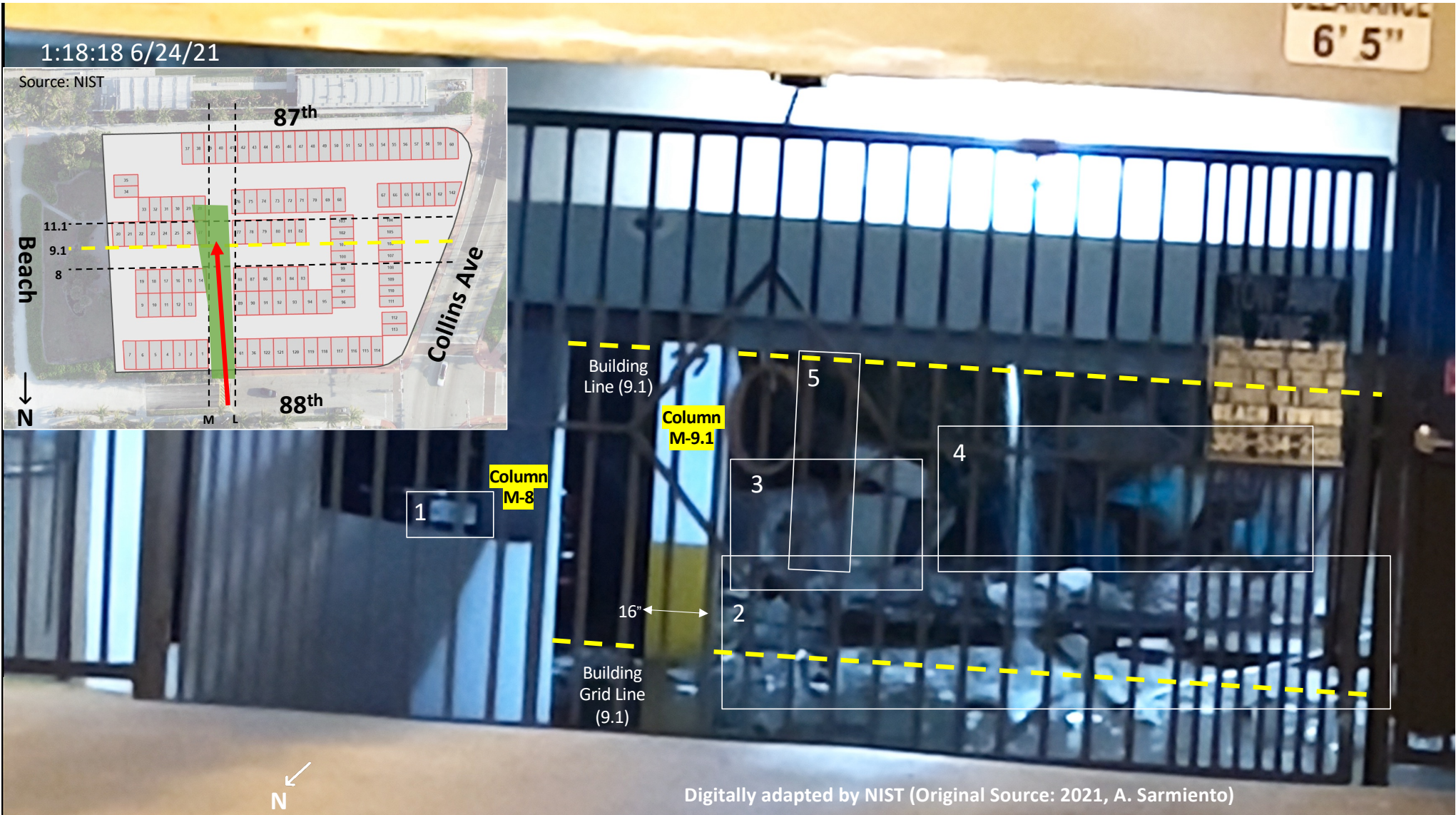
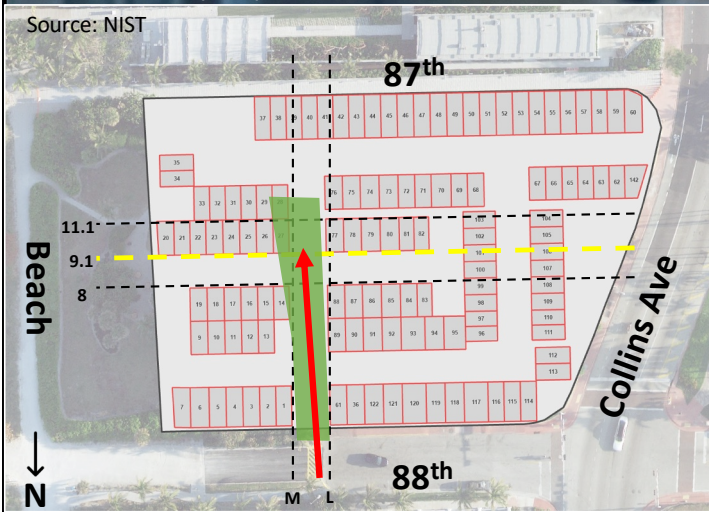
N ↙

Digitally adapted by NIST (Original Source: 2021, A. Sarmiento)



1:18:18 6/24/21

Source: NIST



Digitally adapted by NIST (Original Source: 2021, A. Sarmiento)

# Timeline of Collapse Initiation and Progression: Beach Walkway Video



01:18:18

Visual Evidence Presented in Slides Spans 4 min 14 s

01:21:50

~01:22

01:21:39

01:22:14

01:22:17





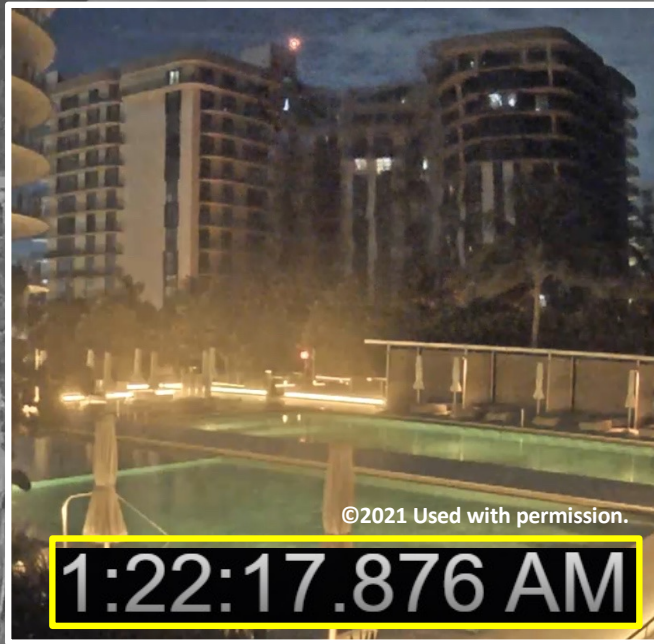
Beach Access Walkway  
camera activated by  
motion

1:21:00 1:21:39 1:22:00 1:22:50 1:23:00 1:23:50 1:24:00 1:24:50 1:25:00 1:25:50 1:26:00 1:26:50 1:27:00

N.W. East View 1:21:39.128 AM 6/24/2021

©2021 Used with permission

Key Point: the Beach Access Walkway video captures moments just before, during, and after the tower collapses



Beach Access Walkway camera activated by motion

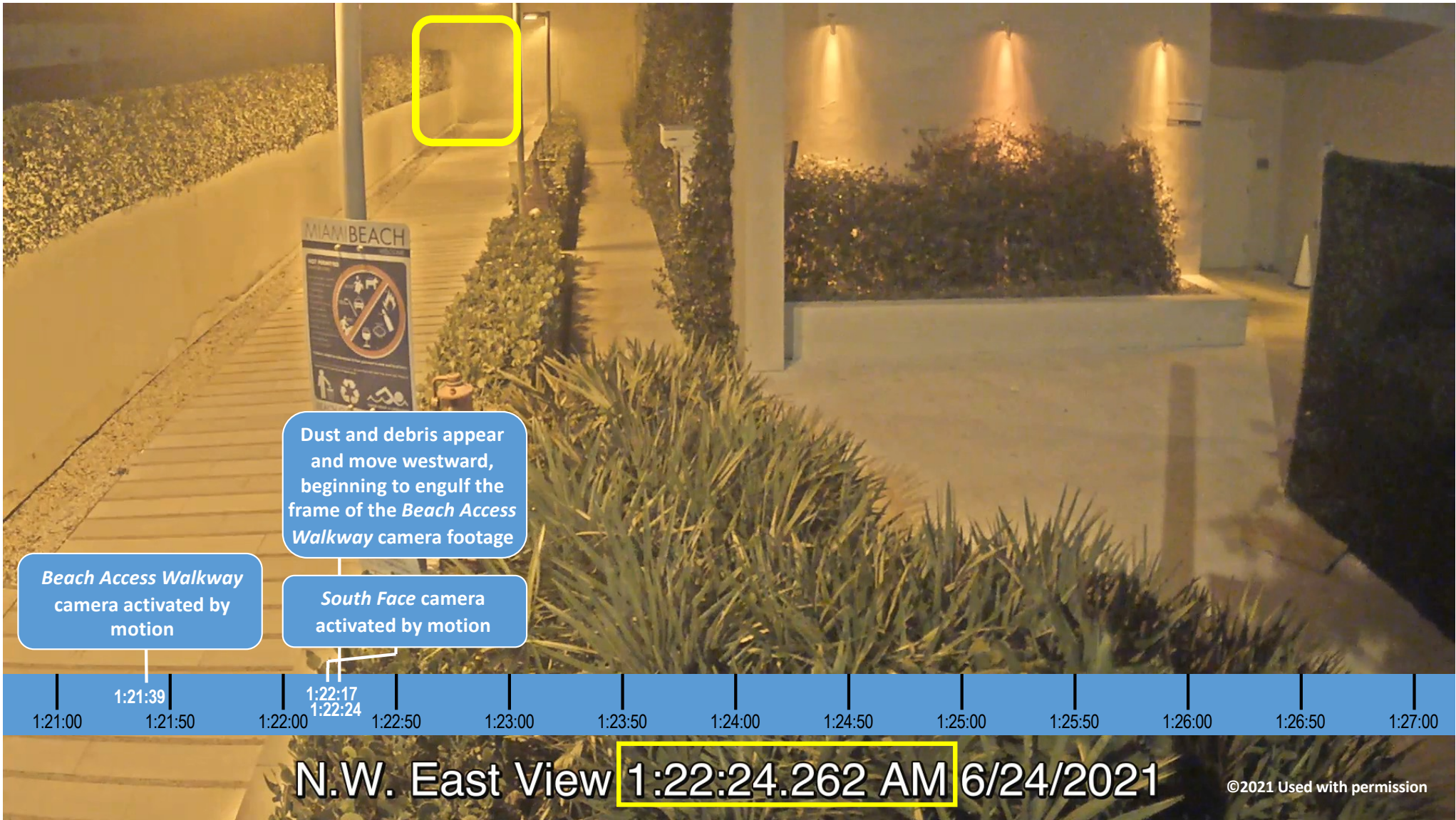
South Face camera activated by motion

1:21:00 1:21:39 1:21:50 1:22:00 1:22:17 1:22:50 1:23:00 1:23:50 1:24:00 1:24:50 1:25:00 1:25:50 1:26:00 1:26:50 1:27:00

N.W. East View 1:21:39.128 AM 6/24/2021

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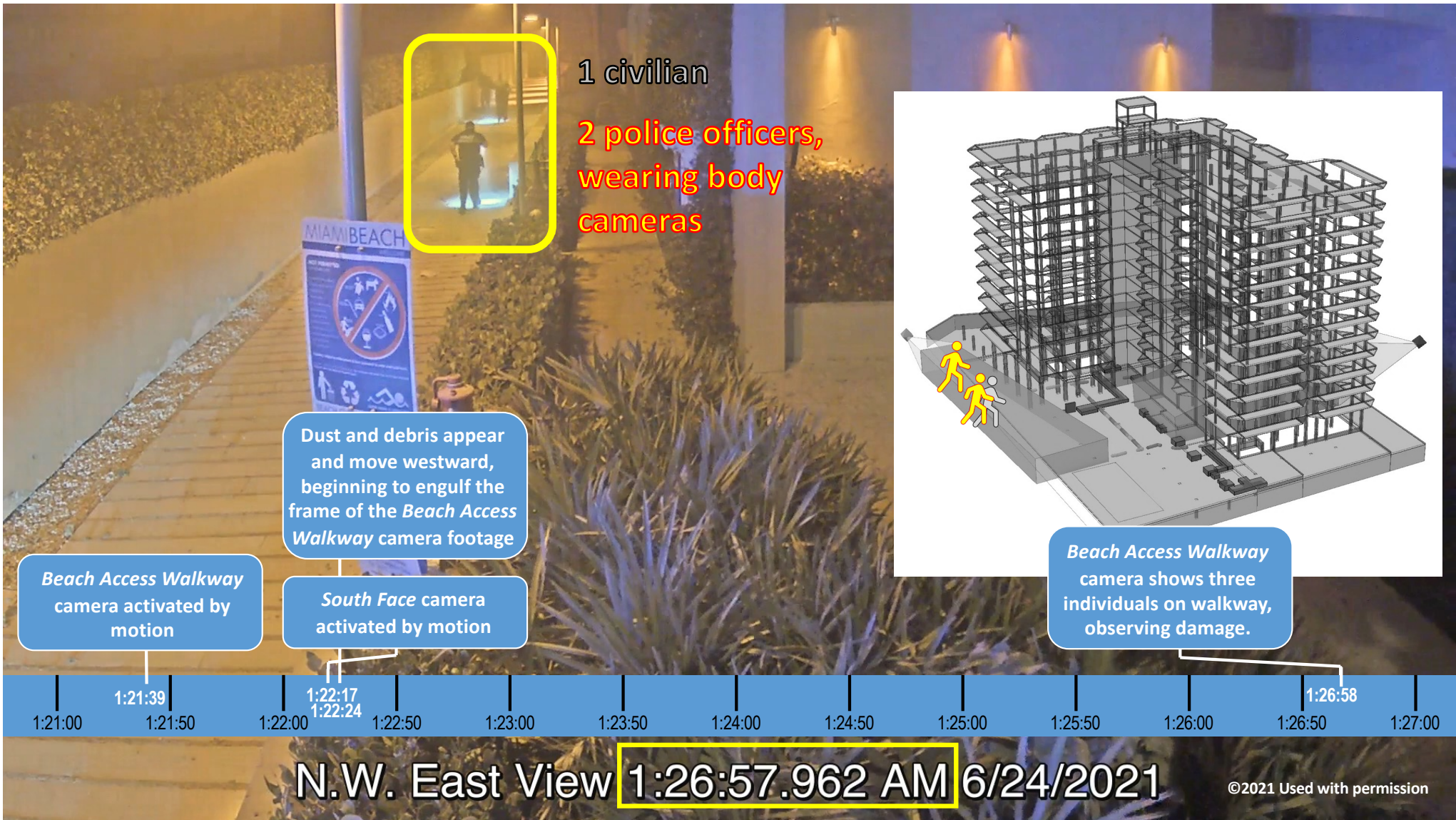
Dust and debris appear and move westward, beginning to engulf the frame of the *Beach Access Walkway* camera footage

*Beach Access Walkway* camera activated by motion

*South Face* camera activated by motion



N.W. East View **1:22:24.262 AM** 6/24/2021



1 civilian

2 police officers,  
wearing body  
cameras

Dust and debris appear  
and move westward,  
beginning to engulf the  
frame of the *Beach Access  
Walkway* camera footage

*Beach Access Walkway*  
camera activated by  
motion

*South Face* camera  
activated by motion

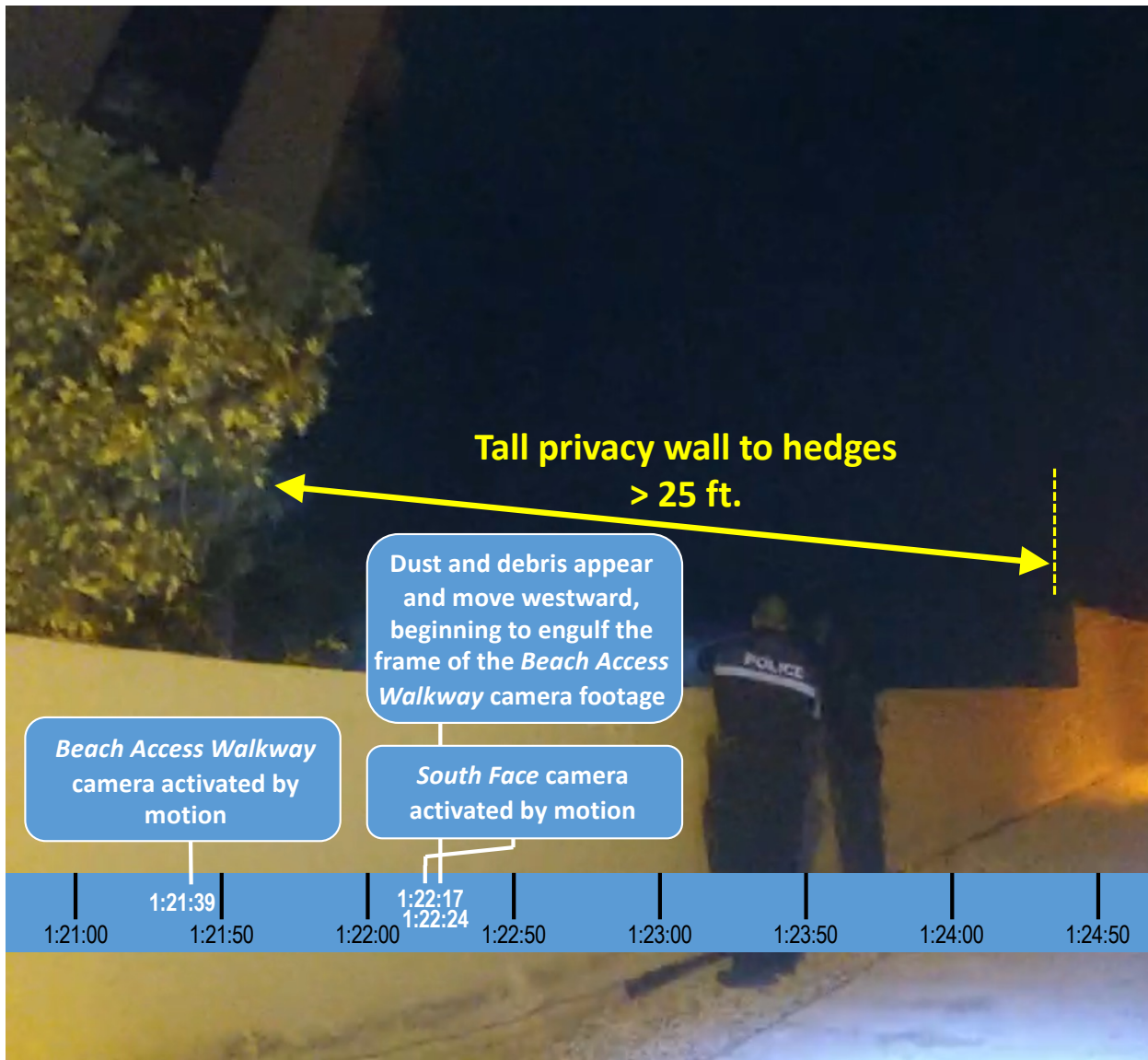
*Beach Access Walkway*  
camera shows three  
individuals on walkway,  
observing damage.



N.W. East View **1:26:57.962 AM** 6/24/2021

©2021 Used with permission

2021-06-24 01:27:06 -0400  
AXON BODY 3 X6030815T

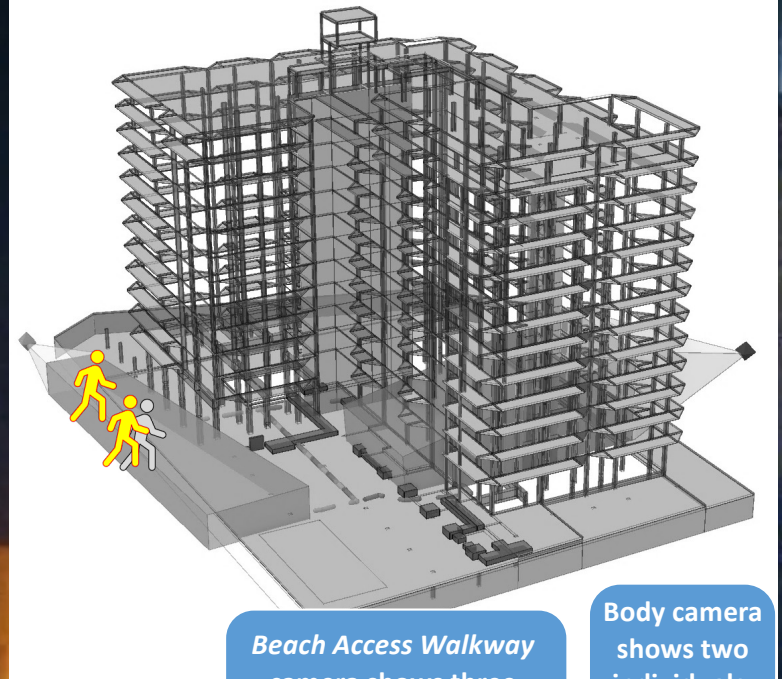


Tall privacy wall to hedges  
> 25 ft.

Dust and debris appear and move westward, beginning to engulf the frame of the Beach Access Walkway camera footage

South Face camera activated by motion

Beach Access Walkway camera activated by motion



Beach Access Walkway camera shows three individuals on walkway, observing damage.

Body camera shows two individuals, observing damage.



# Comparison to Beach Access Walkway Footage

Key Point: Portion of hedges and the metal fence are not visible in Beach Access Walkway footage because they had already collapsed with pool deck



# Timeline of Collapse Initiation and Progression: Upper Story Corridor Video



01:18:18

Visual Evidence Presented in Slides Spans 4 min 14 s

01:21:50

~01:22

01:21:39

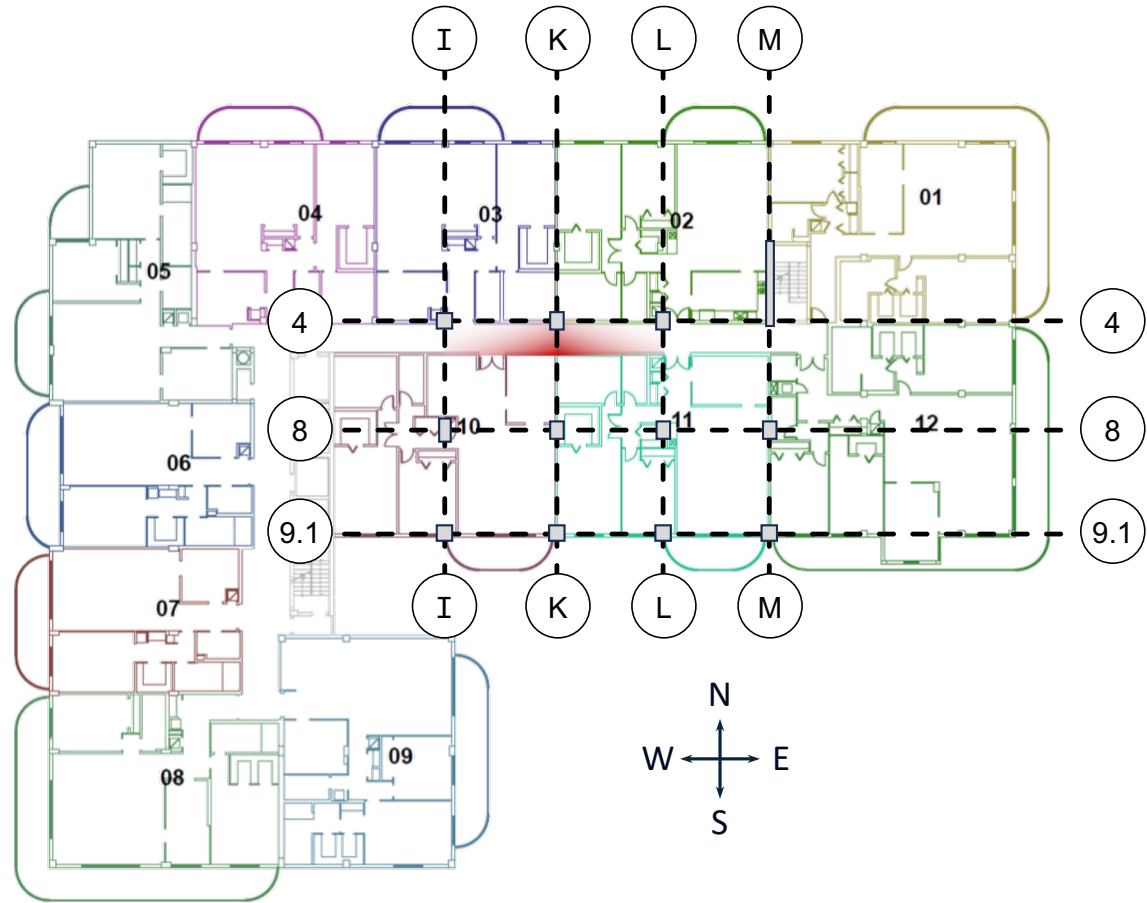
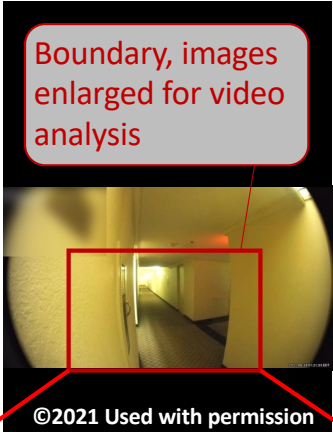
01:22:14

01:22:17





Camera activated by motion

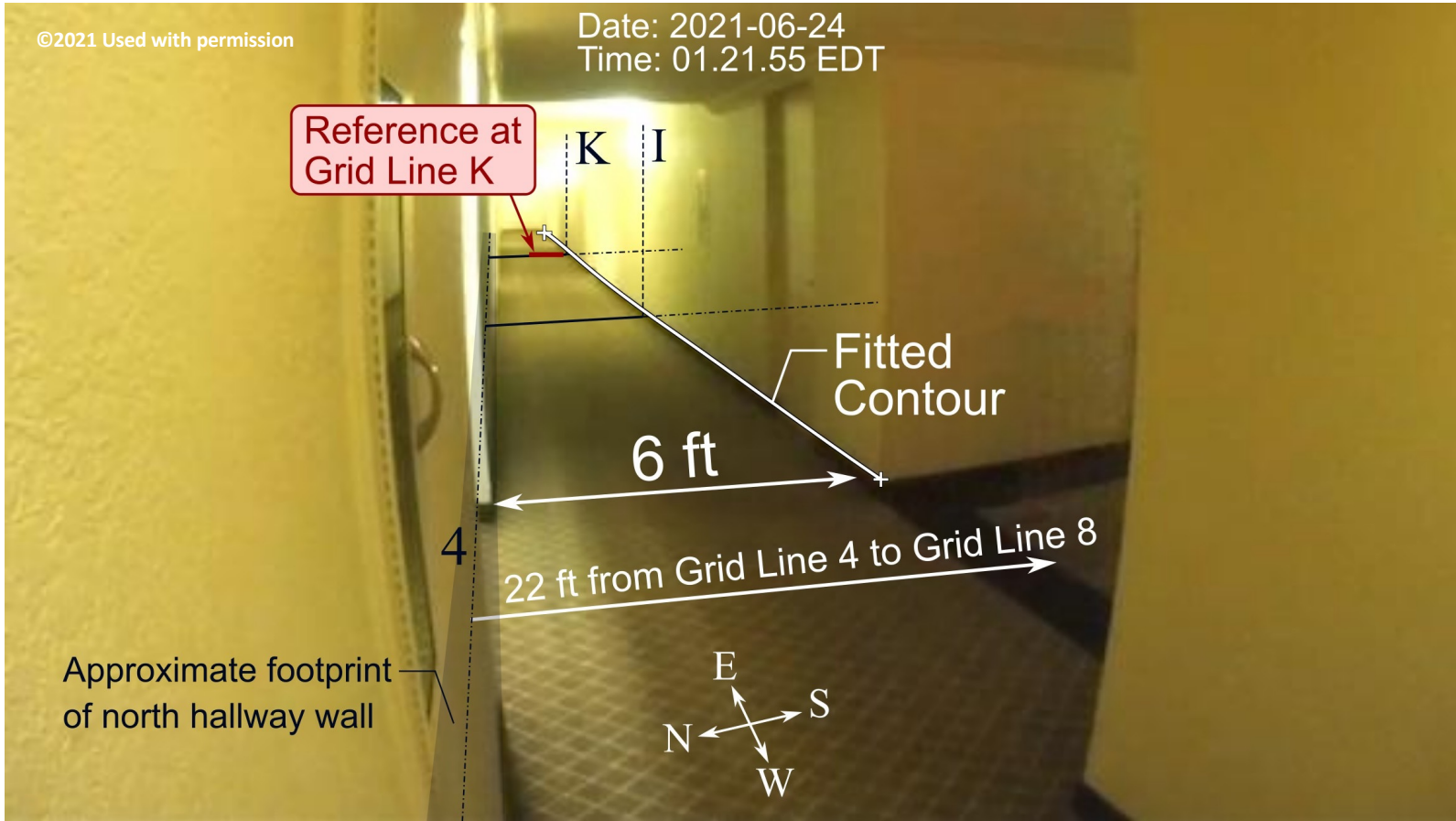


Source: NIST



Camera activated by motion

Initial frame extracted for comparative analysis





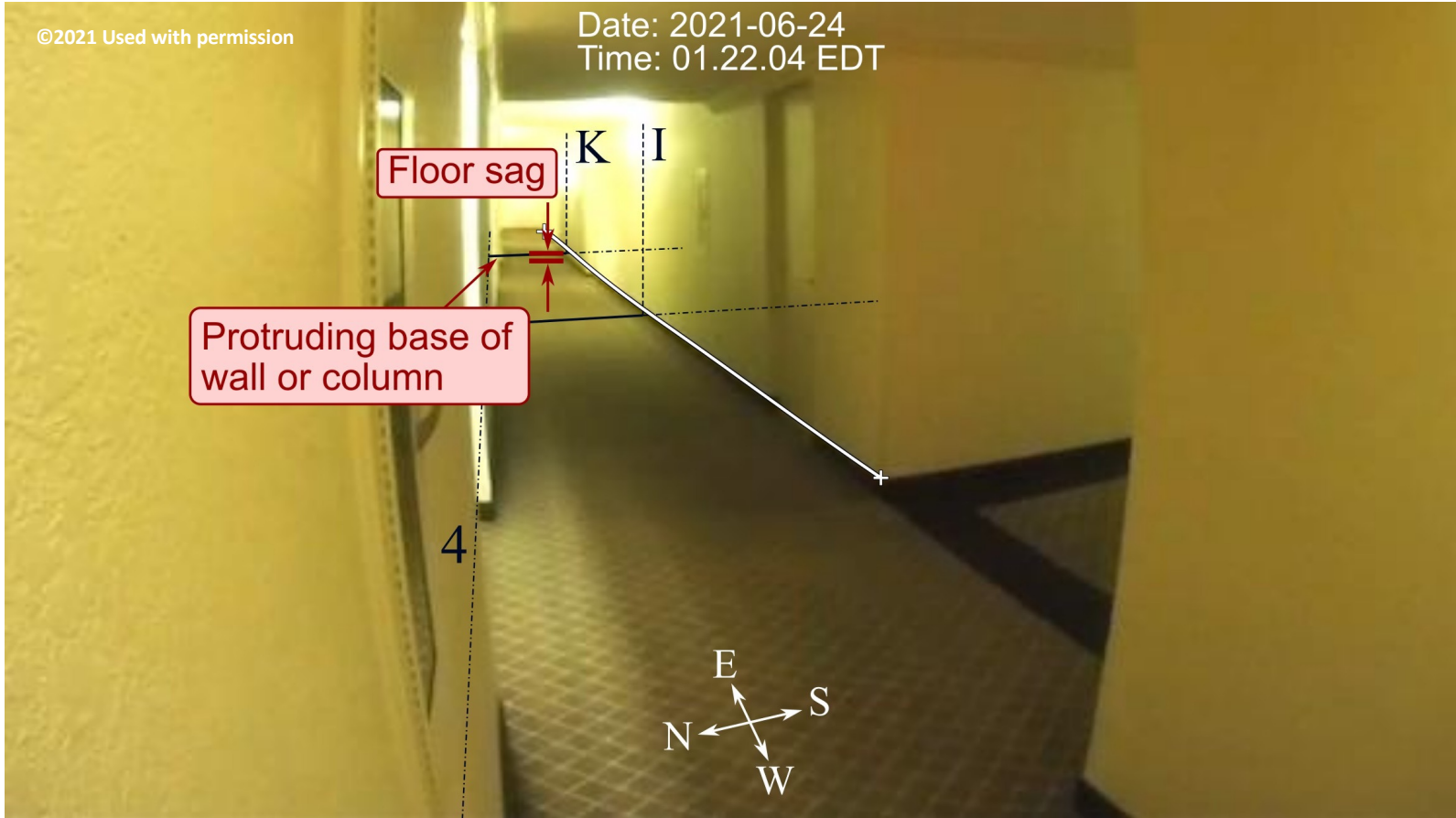
Camera activated by motion

Initial frame extracted for comparative analysis

Begin roaring, rumbling sounds

Floor begins to sag near Grid Line K

Begin displ. at north wall at or near Column K-4







Camera activated by motion

Initial frame extracted for comparative analysis

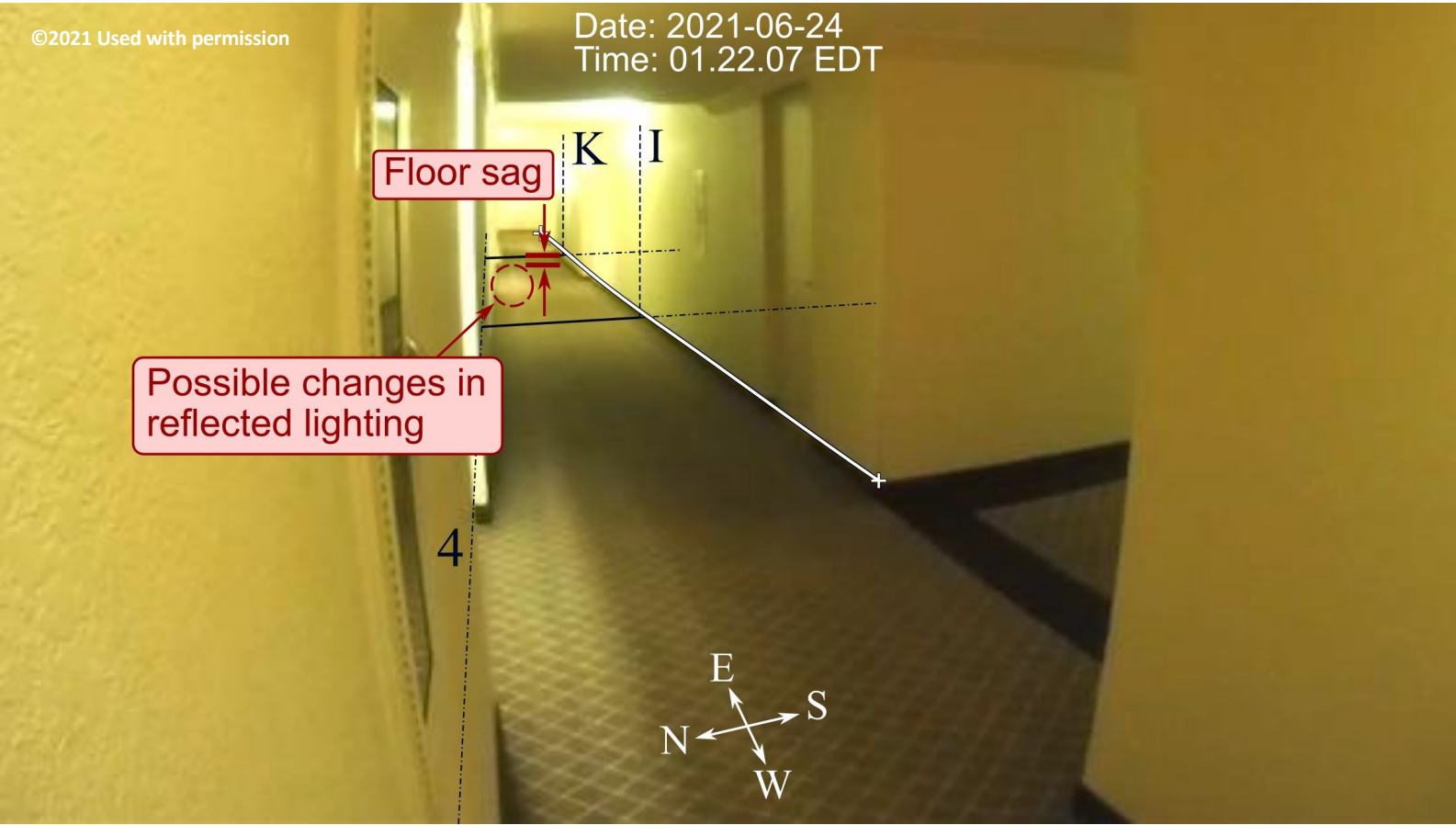
Begin roaring, rumbling sounds

Floor begins to sag near Grid Line K

Begin displ. at north wall at or near Column K-4

Possible change in floor angle west of Grid Line K

Floor movement briefly slows





Camera activated by motion

Initial frame extracted for comparative analysis

Begin roaring, rumbling sounds

Floor begins to sag near Grid Line K

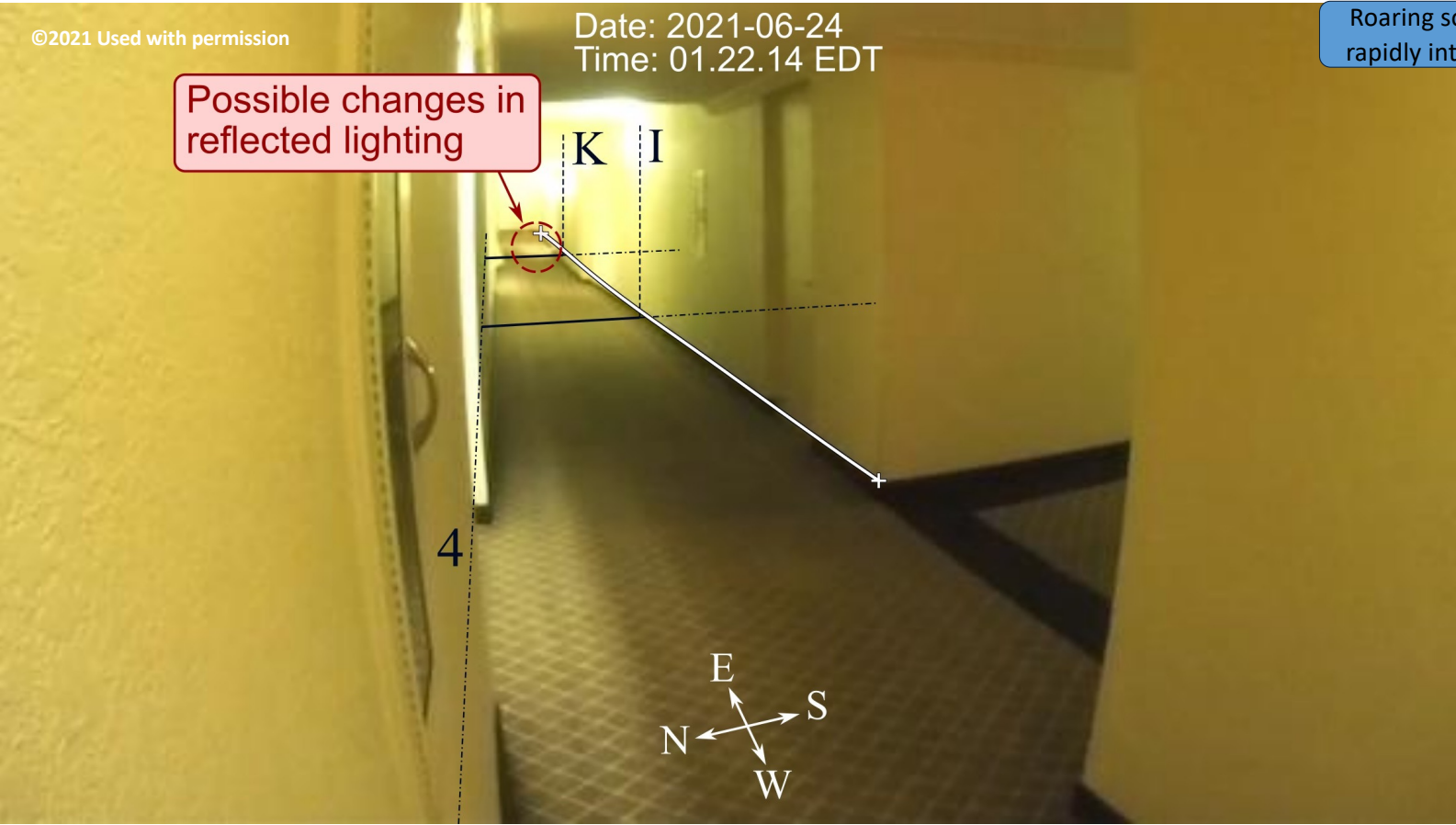
Begin displ. at north wall at or near Column K-4

Possible change in floor angle west of Grid Line K

Floor movement briefly slows

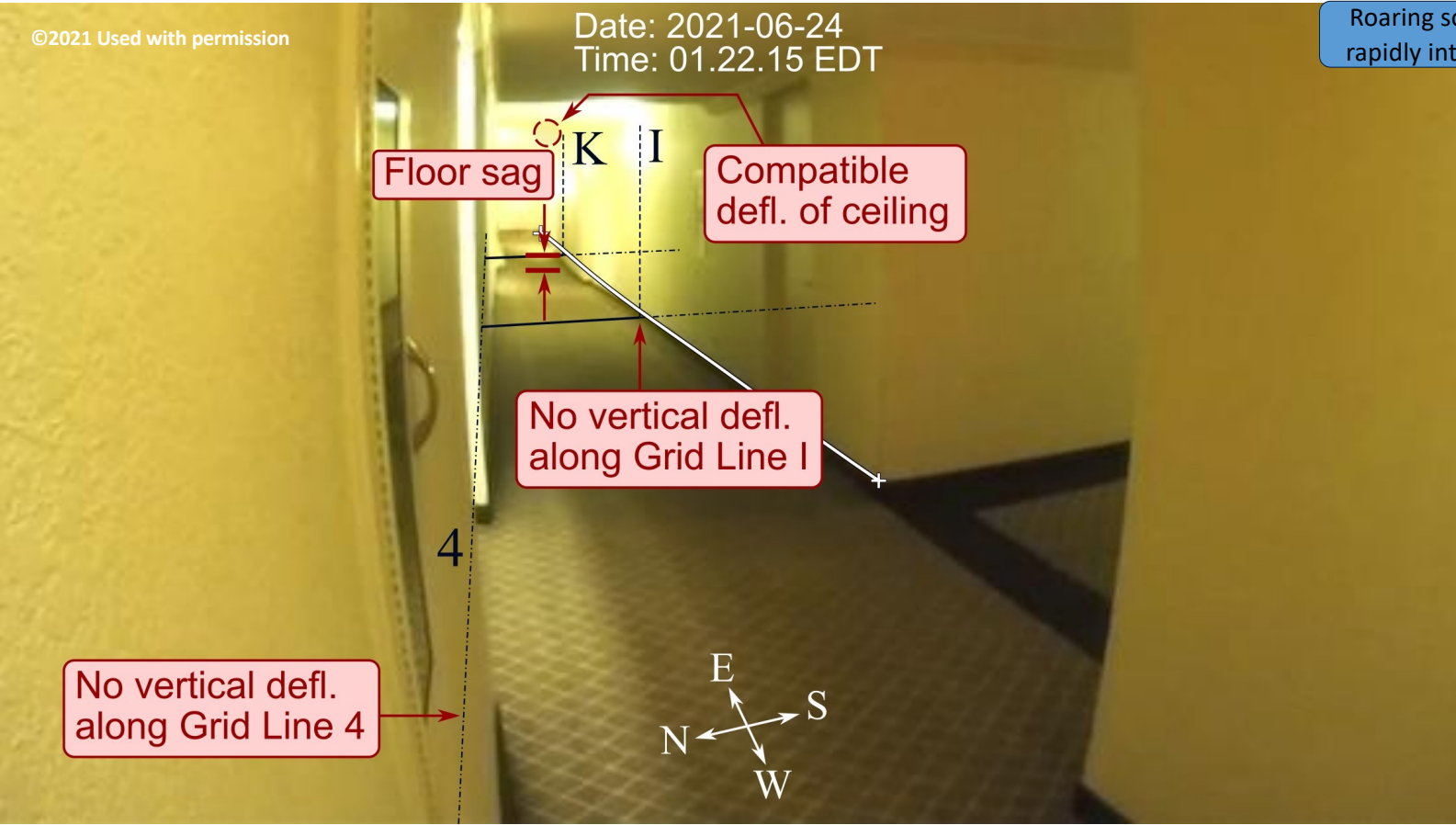
Possible change in floor angle east of Grid Line K

Roaring sounds rapidly intensify





- 1:21:49 - Camera activated by motion
- 1:21:55 - Initial frame extracted for comparative analysis
- 1:21:58 - Begin roaring, rumbling sounds
- 1:22:00 - Floor begins to sag near Grid Line K
- 1:22:03 - Begin displ. at north wall at or near Column K-4
- 1:22:07 - Possible change in floor angle west of Grid Line K
- 1:22:11 - Floor movement briefly slows
- 1:22:13 - Possible change in floor angle east of Grid Line K
- 1:22:15 - Roaring sounds rapidly intensify
- 1:22:16 - End of video



# Timeline of Collapse Initiation and Progression: Stack 11 Unit Video



01:18:18

Visual Evidence Presented in Slides Spans 4 min 14 s

01:21:50

~01:22

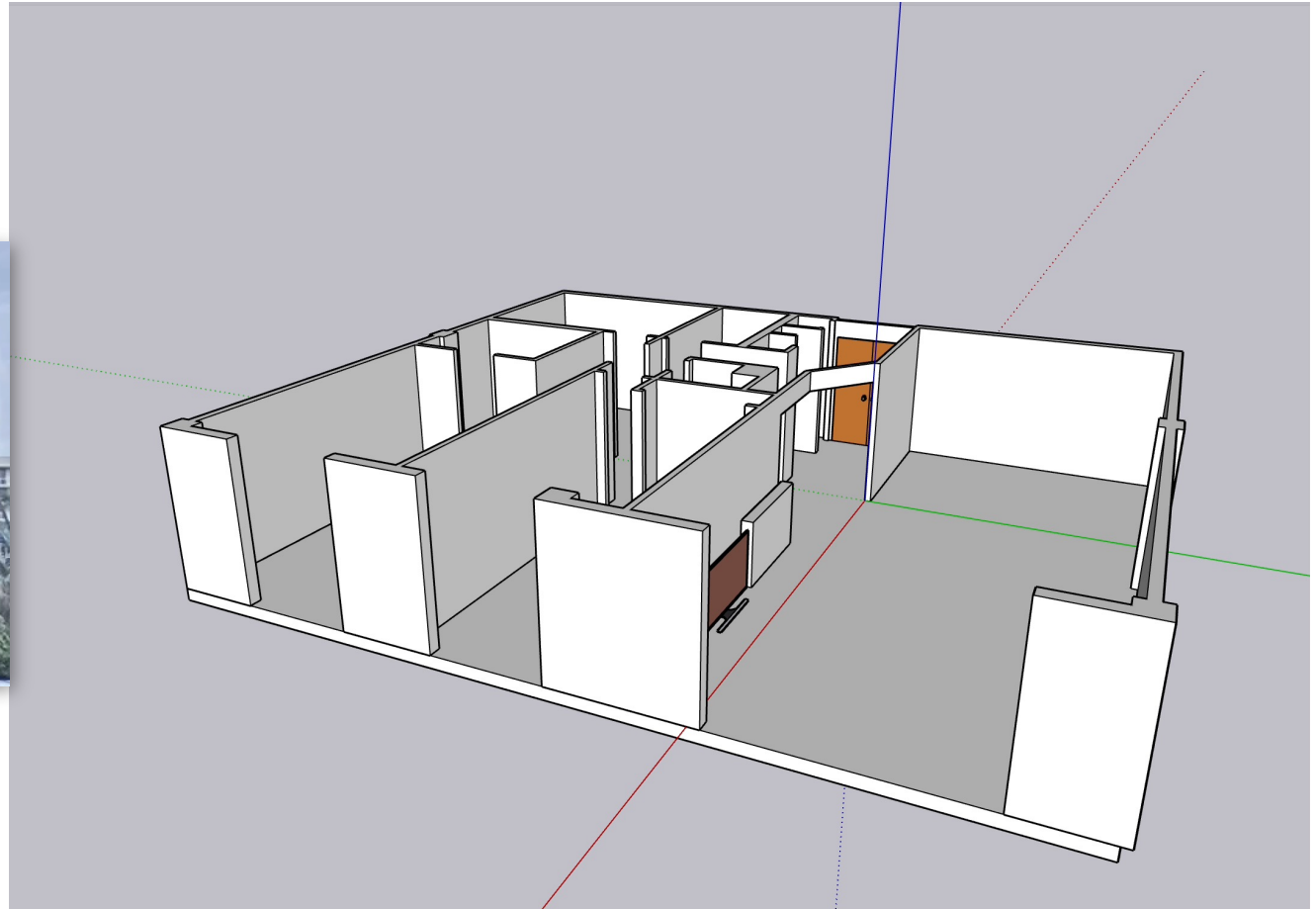
01:21:39

01:22:14

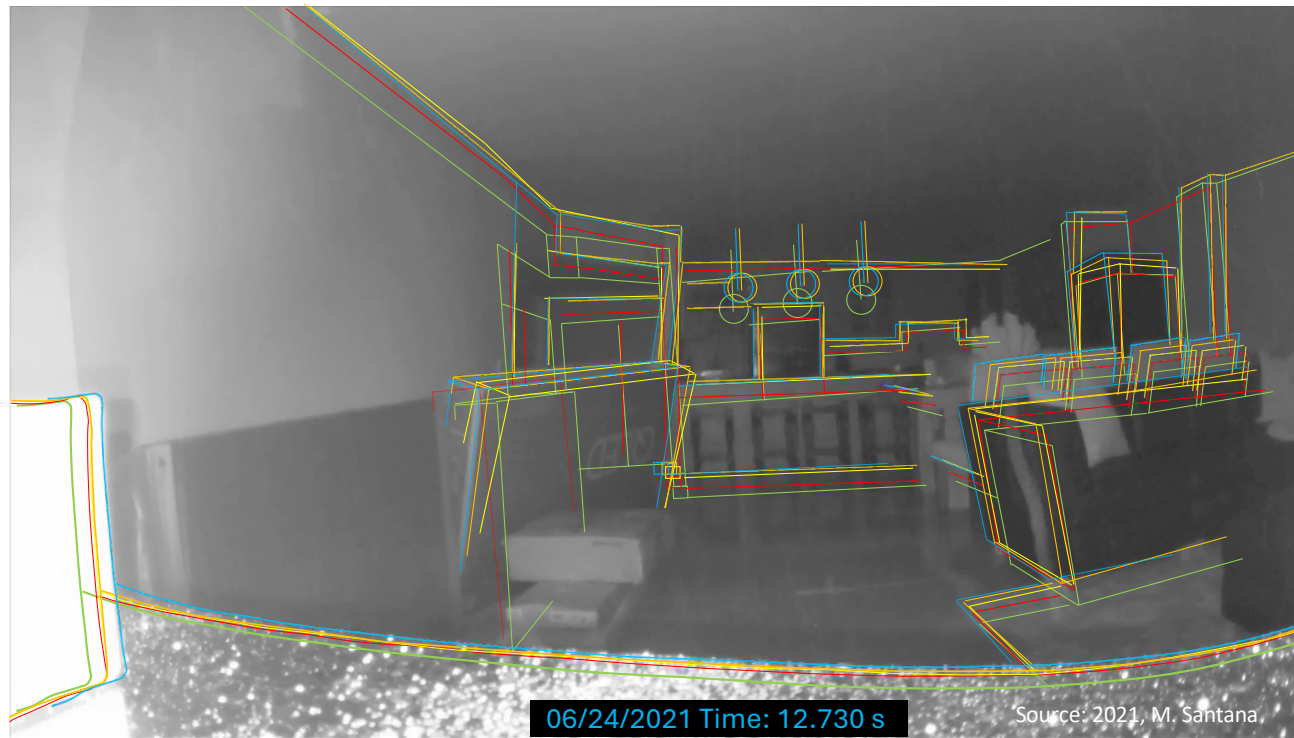
01:22:17



# Position and Orientation of 11 Stack Unit Camera



# Tracking movement of objects in the 11 Stack Unit Video



# Timeline of Collapse Initiation and Progression: South Face Camera Video



01:18:18

Visual Evidence Presented in Slides Spans 4 min 14 s

01:21:50

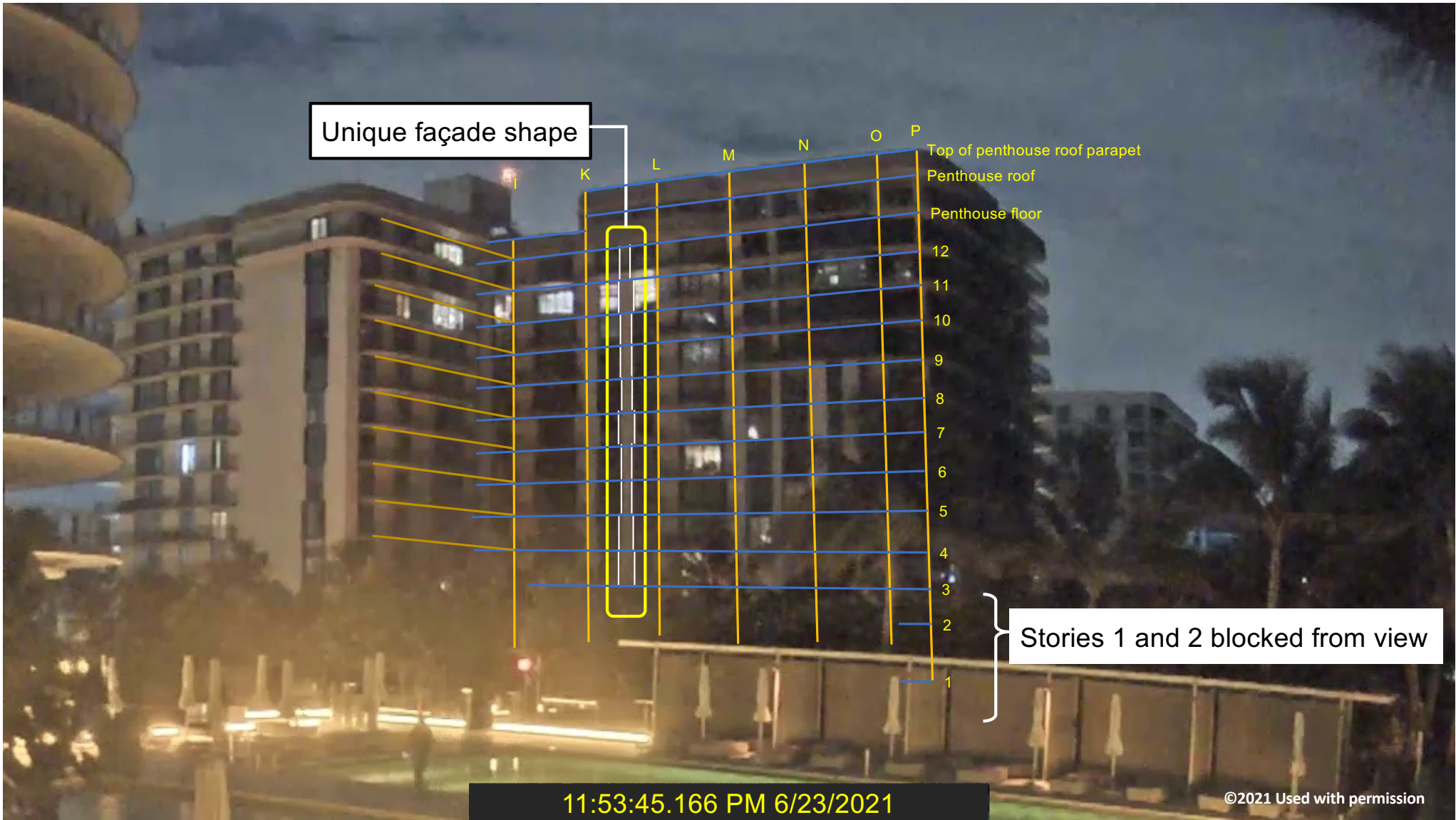
~01:22

01:21:39

01:22:14

01:22:17





Unique façade shape

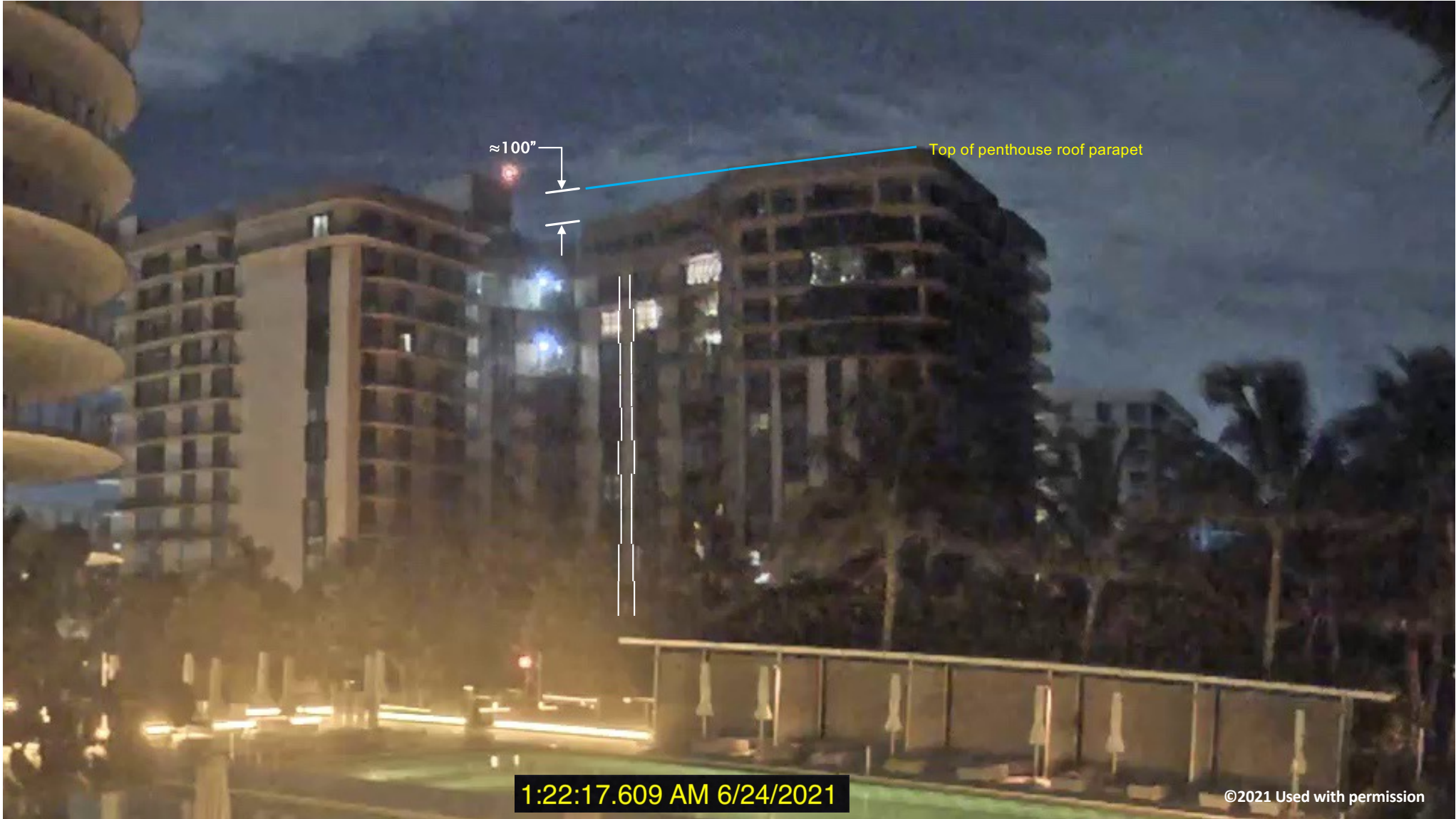
Top of penthouse roof parapet  
Penthouse roof  
Penthouse floor

Stories 1 and 2 blocked from view

11:53:45.166 PM 6/23/2021

©2021 Used with permission



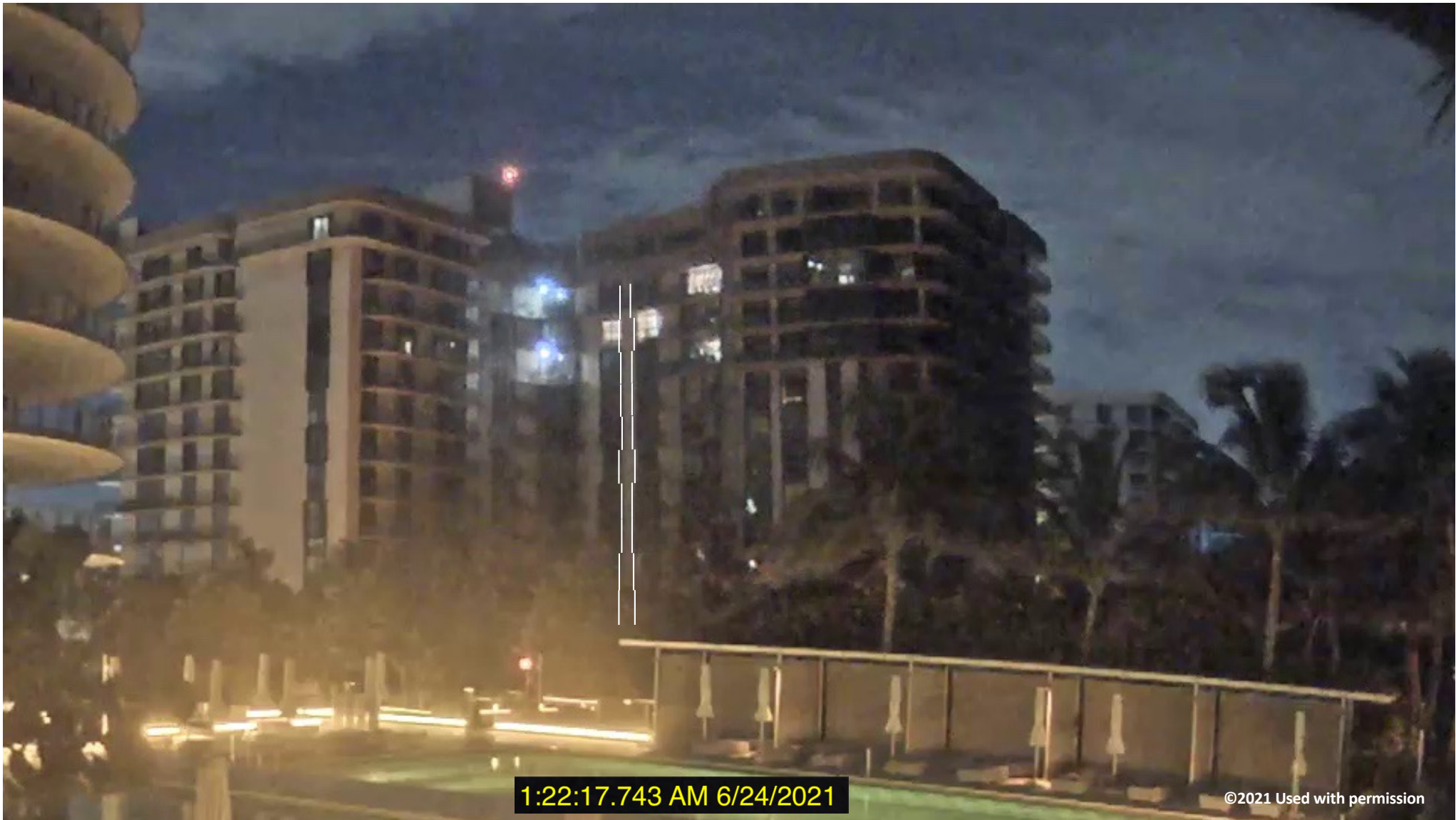


≈ 100"

Top of penthouse roof parapet

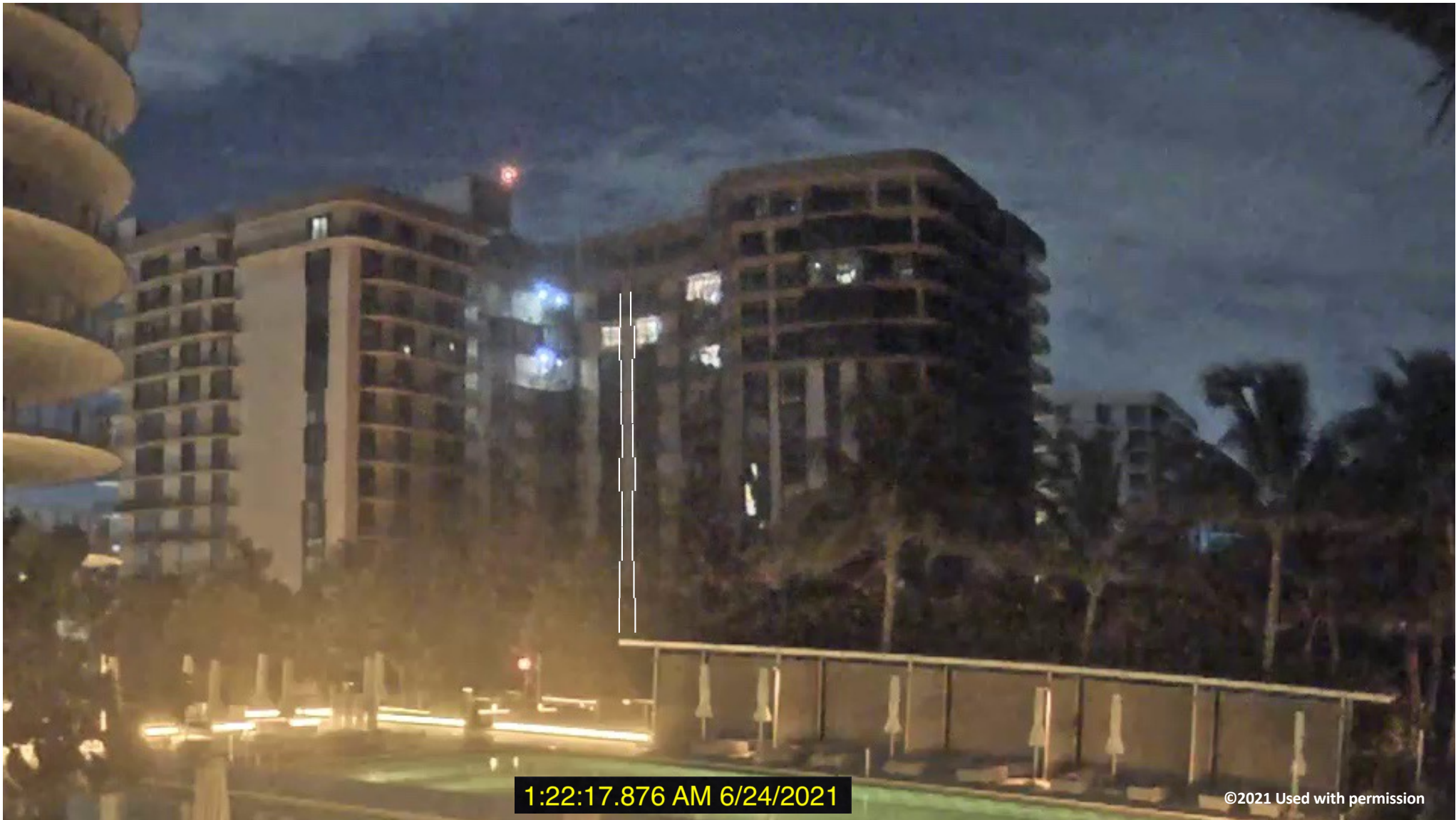
1:22:17.609 AM 6/24/2021

©2021 Used with permission



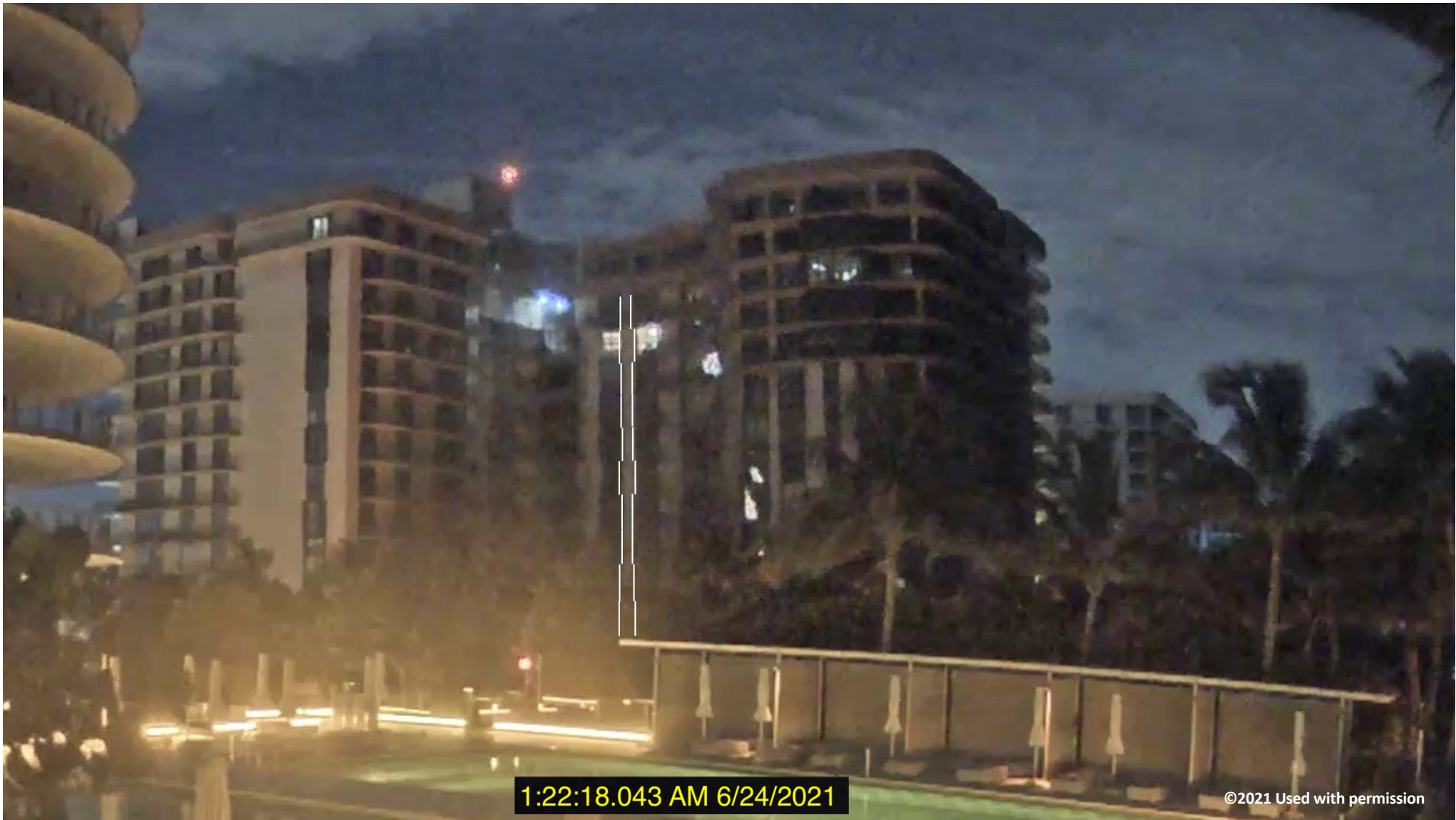
1:22:17.743 AM 6/24/2021

©2021 Used with permission



1:22:17.876 AM 6/24/2021

©2021 Used with permission



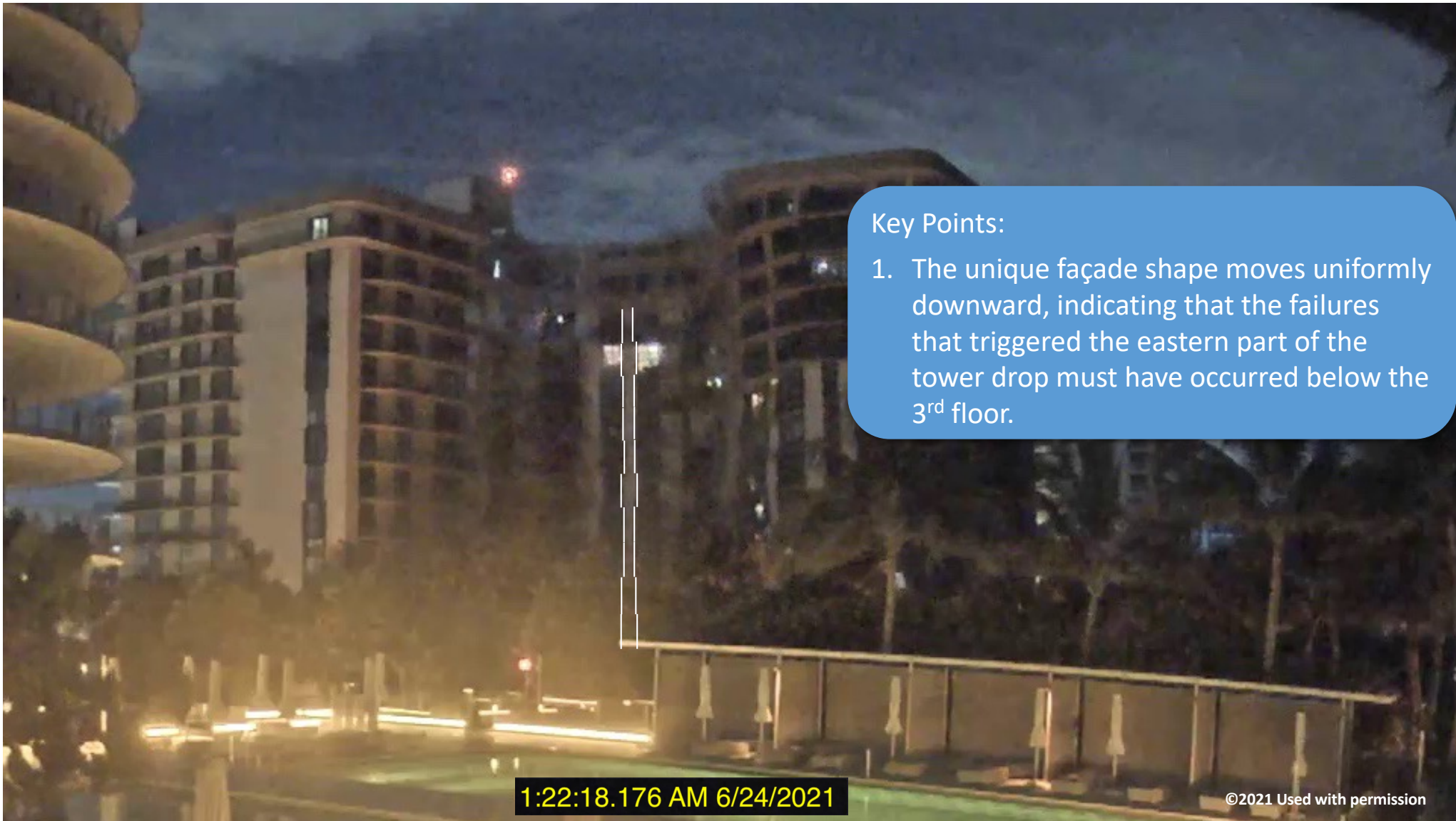
1:22:18.043 AM 6/24/2021

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1:22:18.043+ AM 6/24/2021

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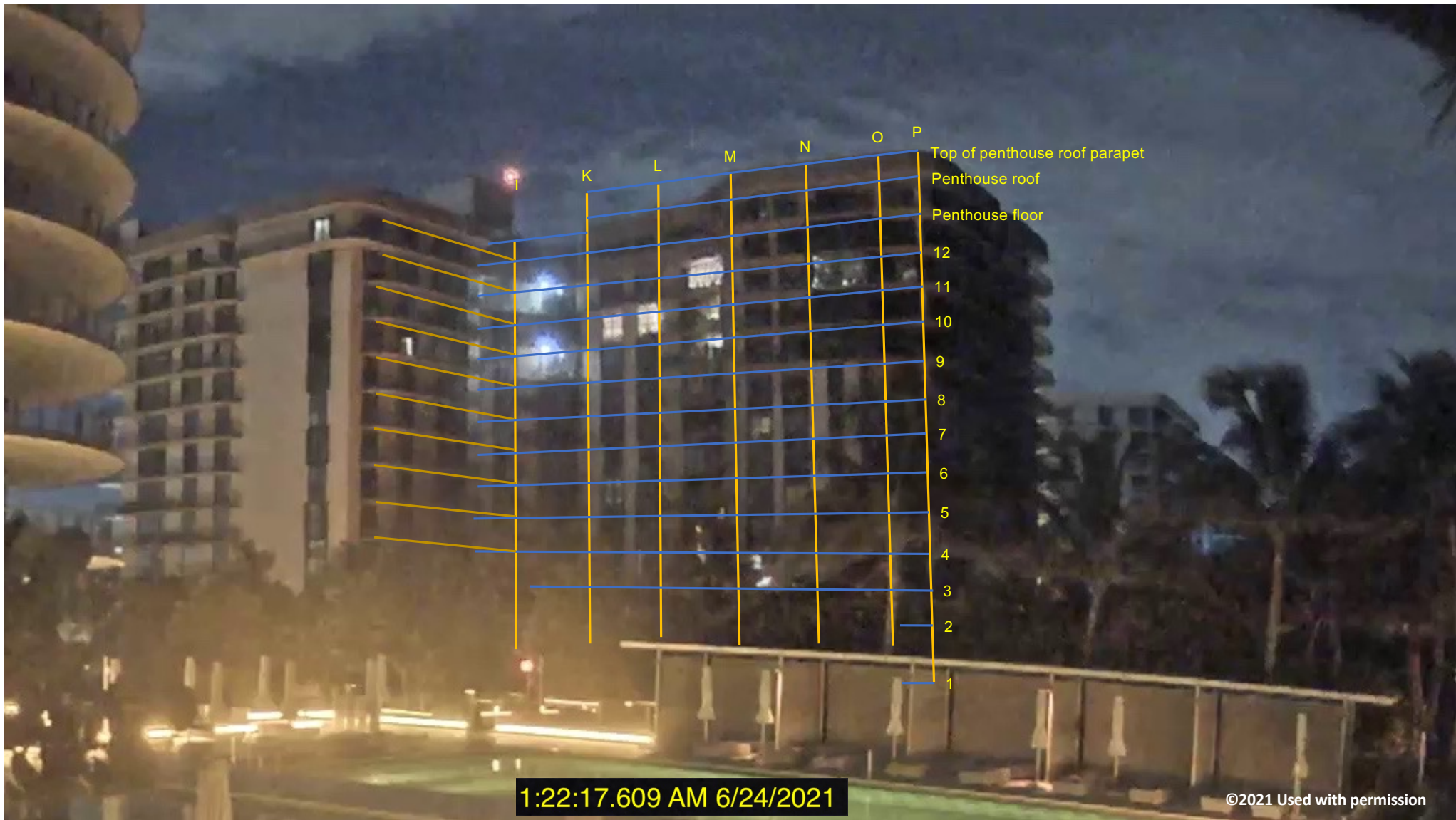


Key Points:

1. The unique façade shape moves uniformly downward, indicating that the failures that triggered the eastern part of the tower drop must have occurred below the 3<sup>rd</sup> floor.

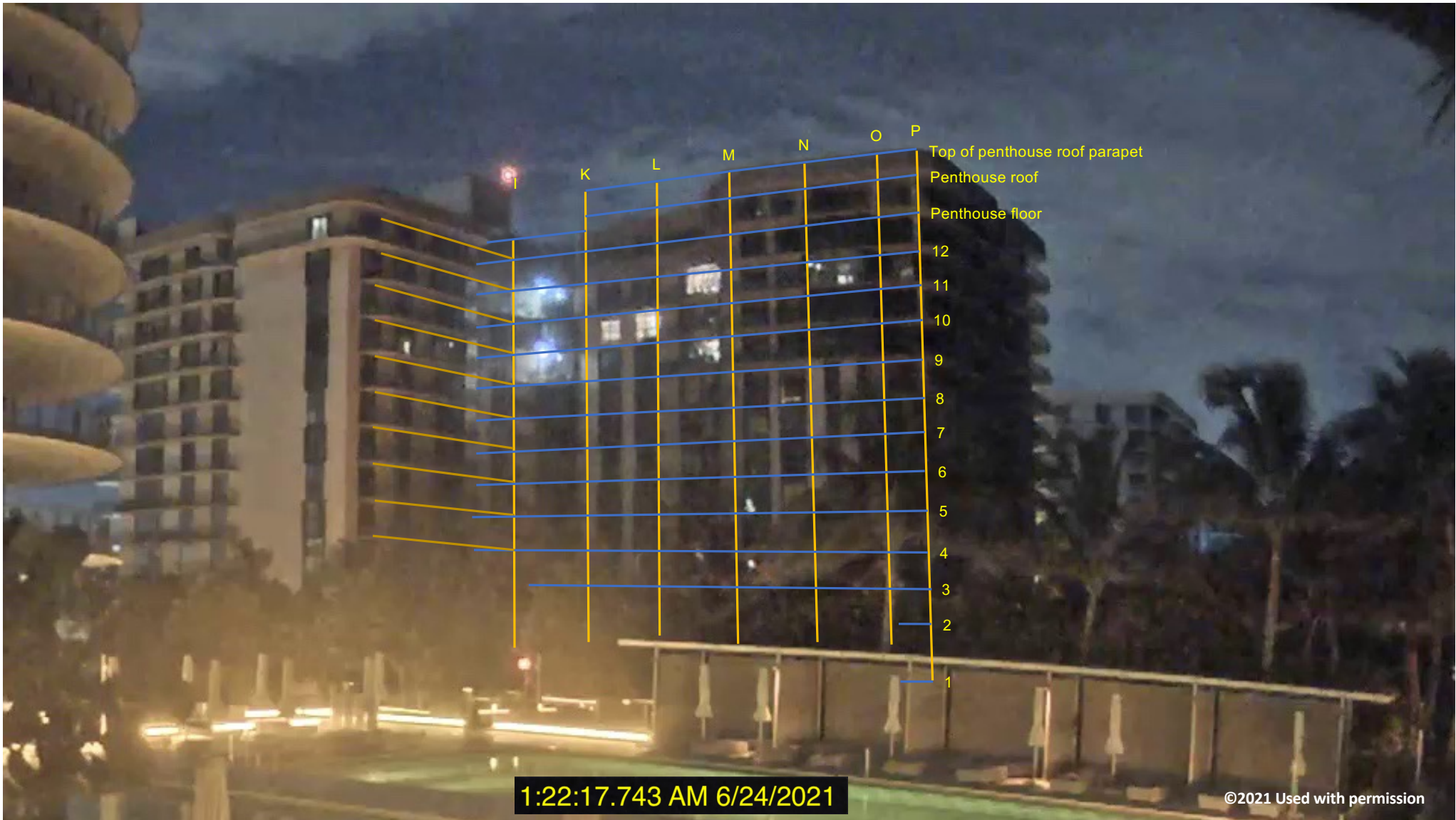
1:22:18.176 AM 6/24/2021

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1:22:17.609 AM 6/24/2021

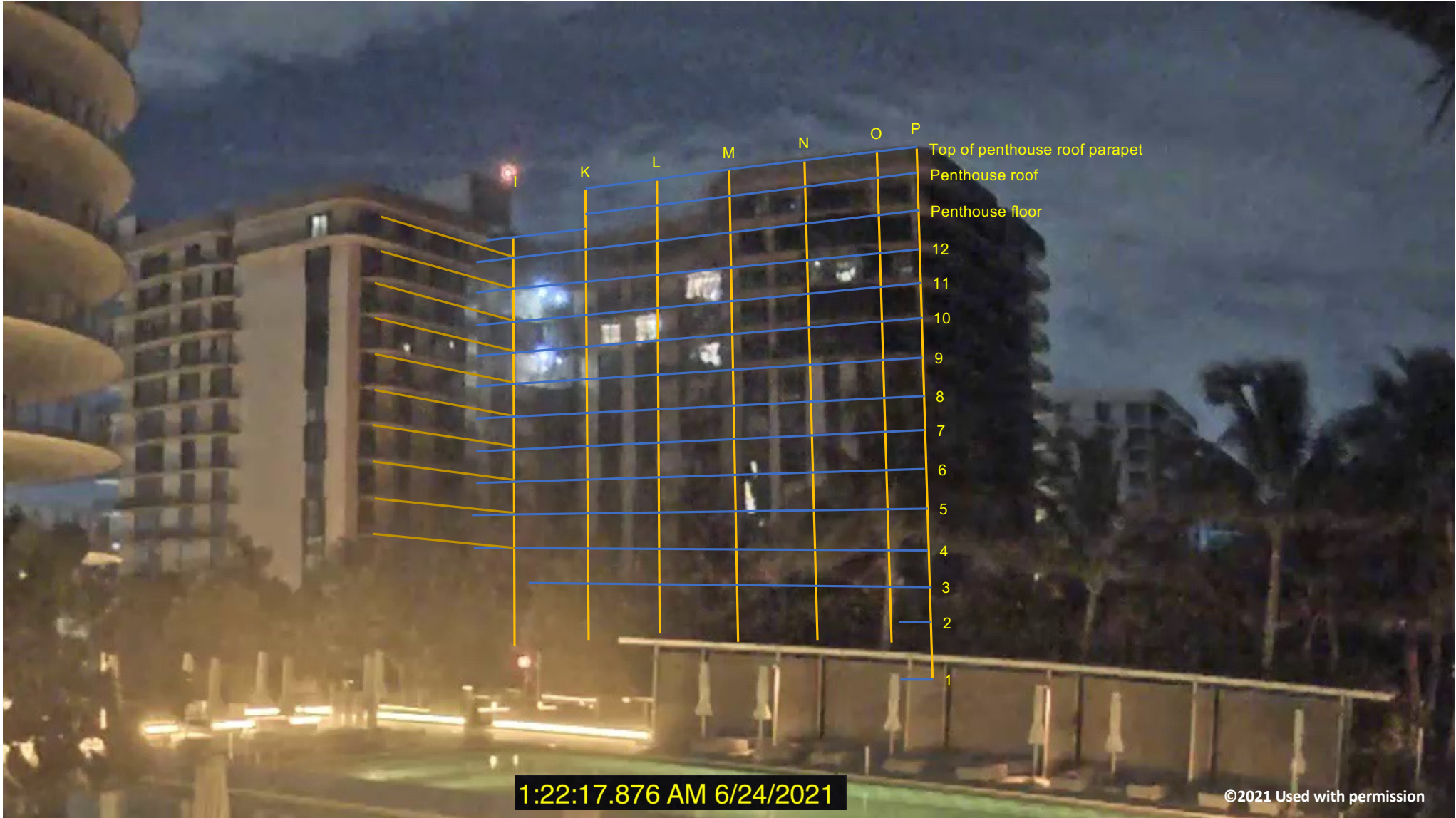
©2021 Used with permission



1:22:17.743 AM 6/24/2021

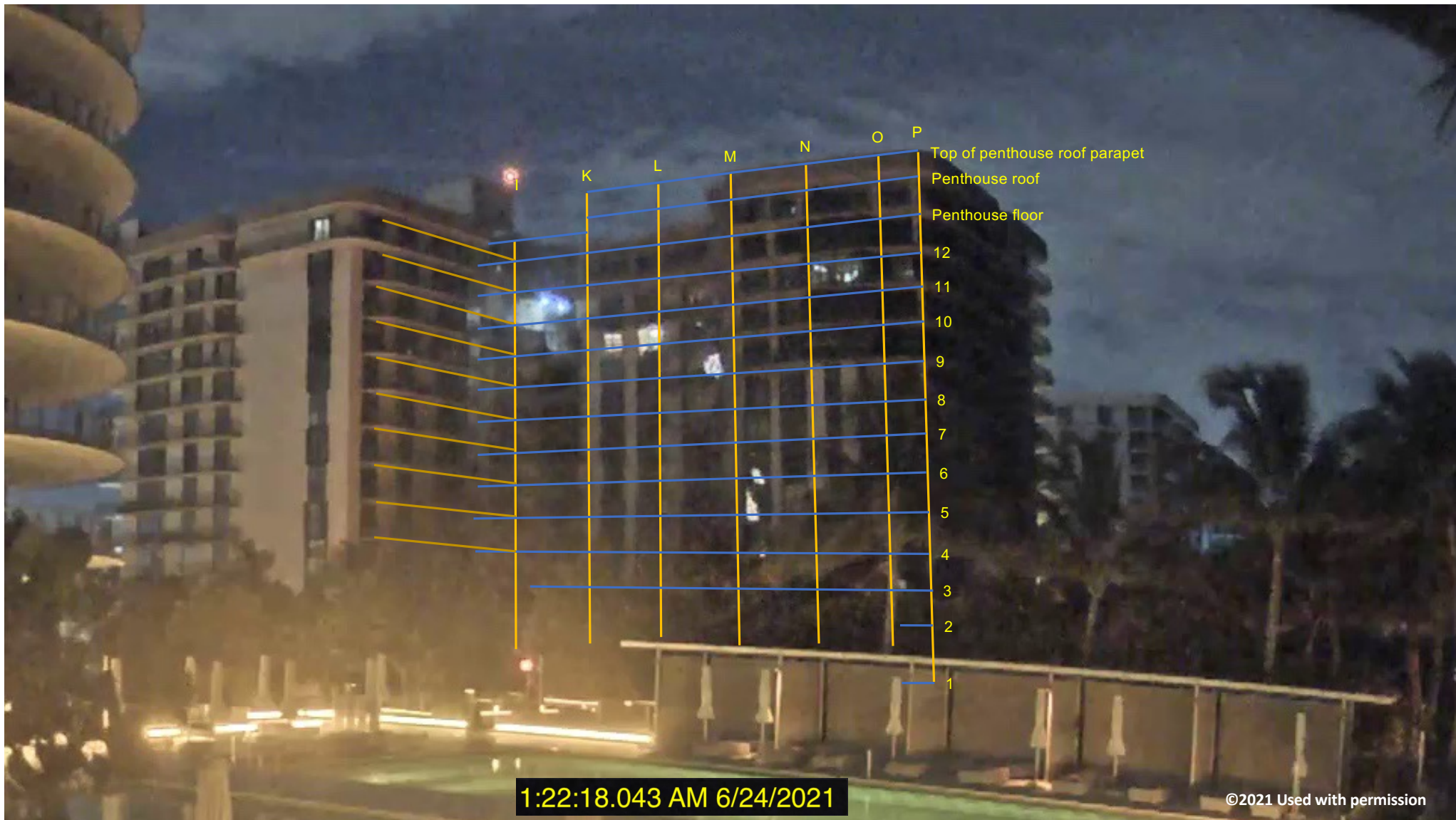
©2021 Used with permission





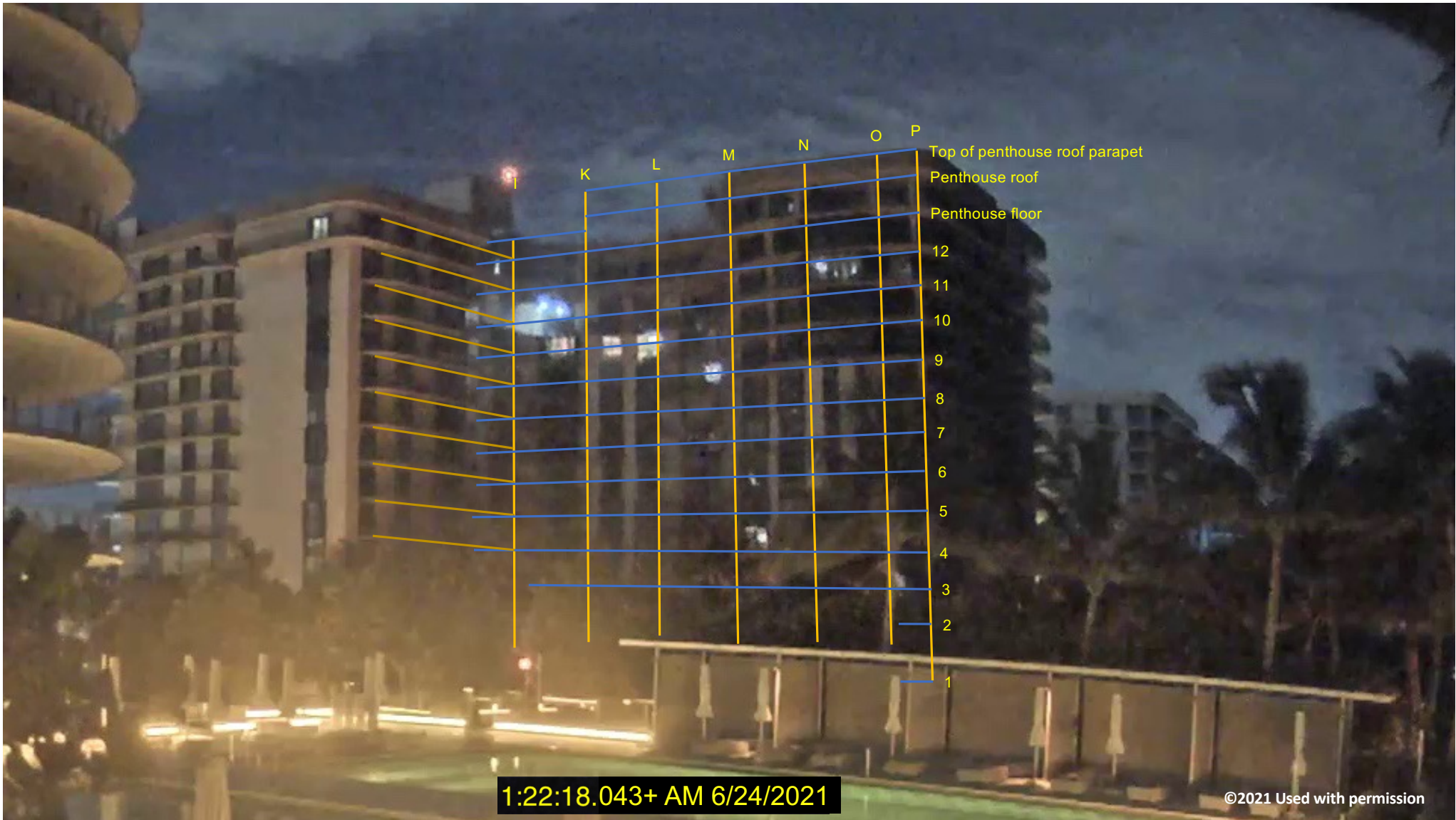
1:22:17.876 AM 6/24/2021

©2021 Used with permission



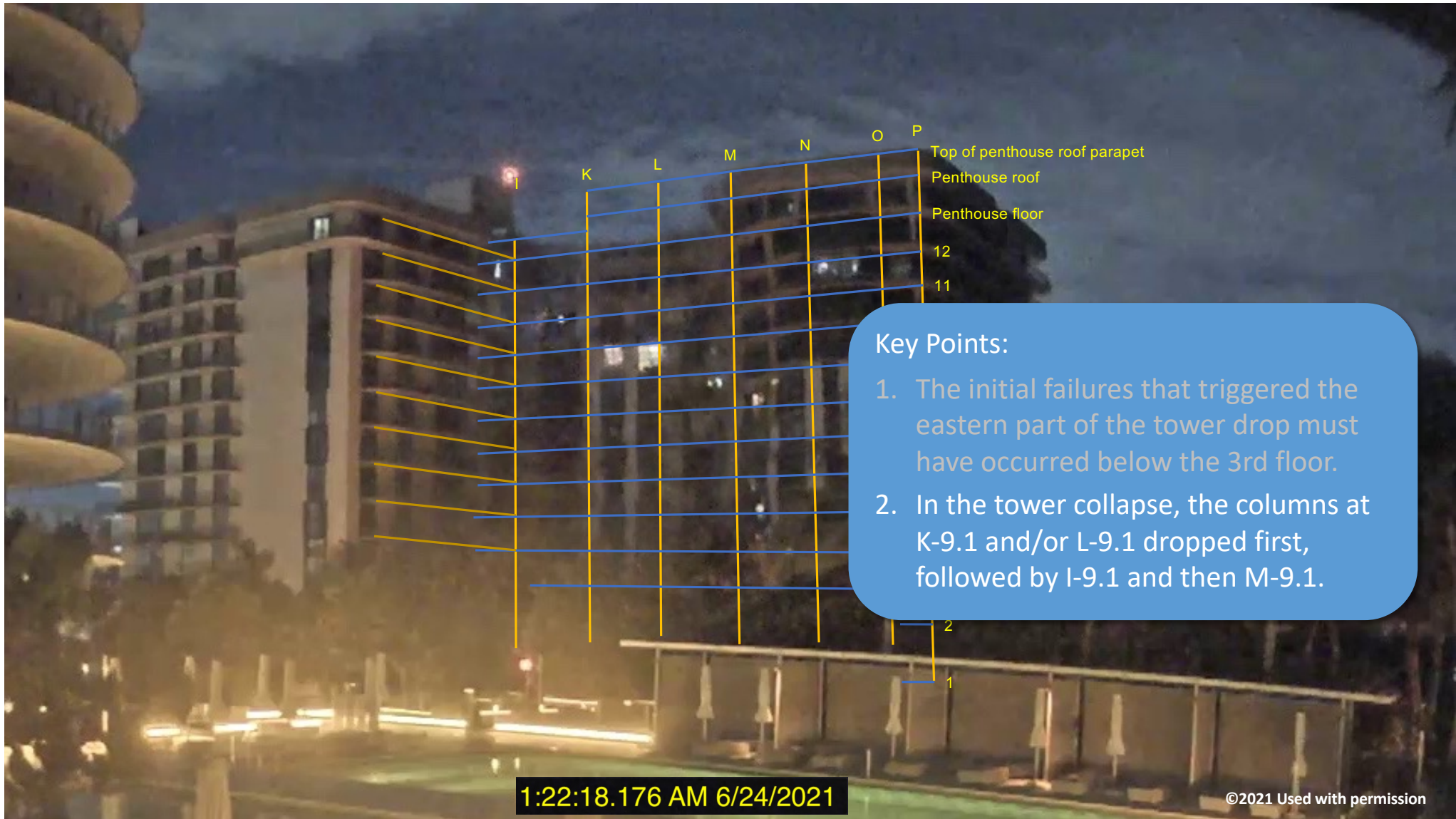
1:22:18.043 AM 6/24/2021

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1:22:18.043+ AM 6/24/2021

©2021 Used with permission



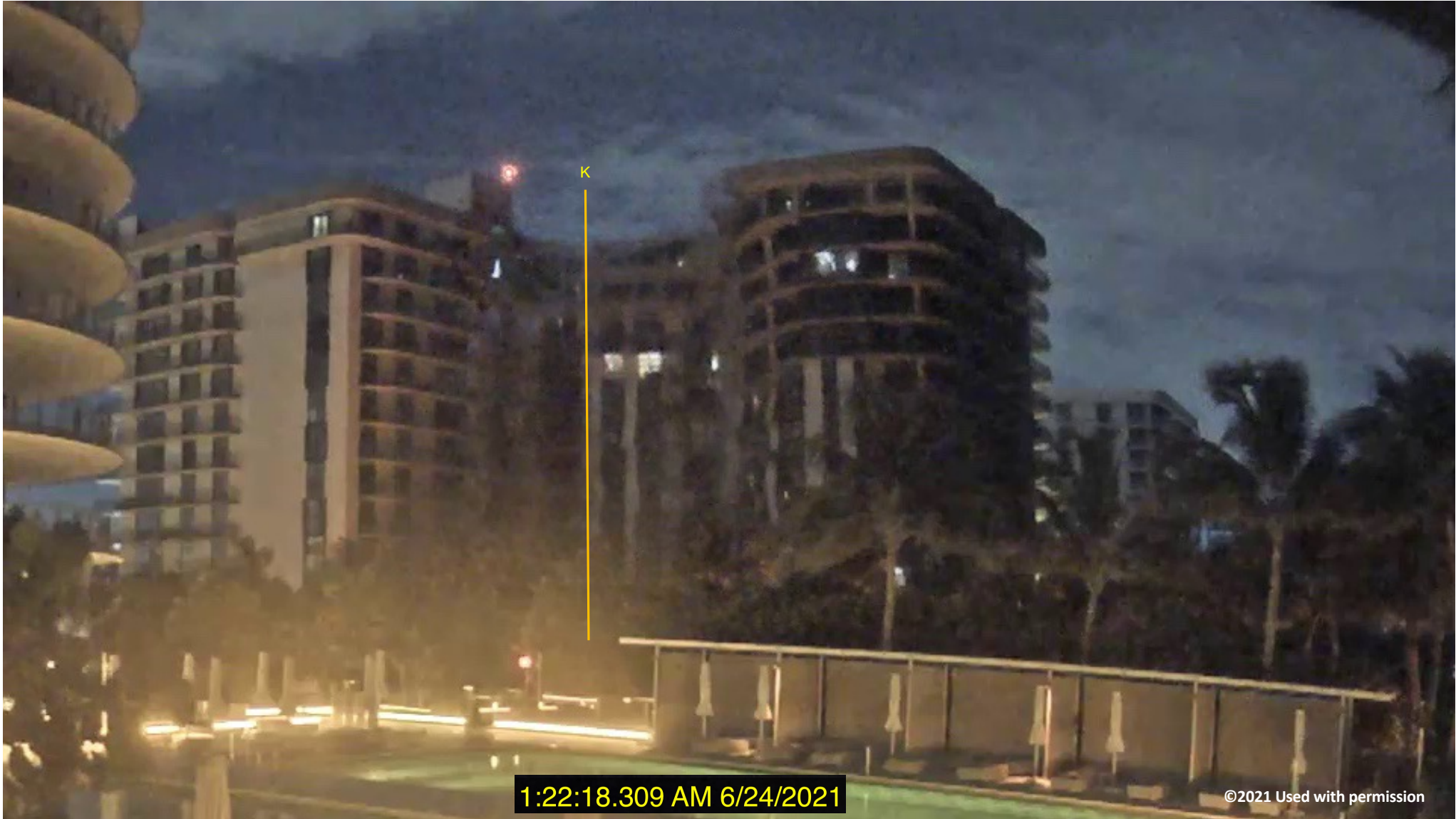
Top of penthouse roof parapet  
Penthouse roof  
Penthouse floor  
12  
11

**Key Points:**

1. The initial failures that triggered the eastern part of the tower drop must have occurred below the 3rd floor.
2. In the tower collapse, the columns at K-9.1 and/or L-9.1 dropped first, followed by I-9.1 and then M-9.1.

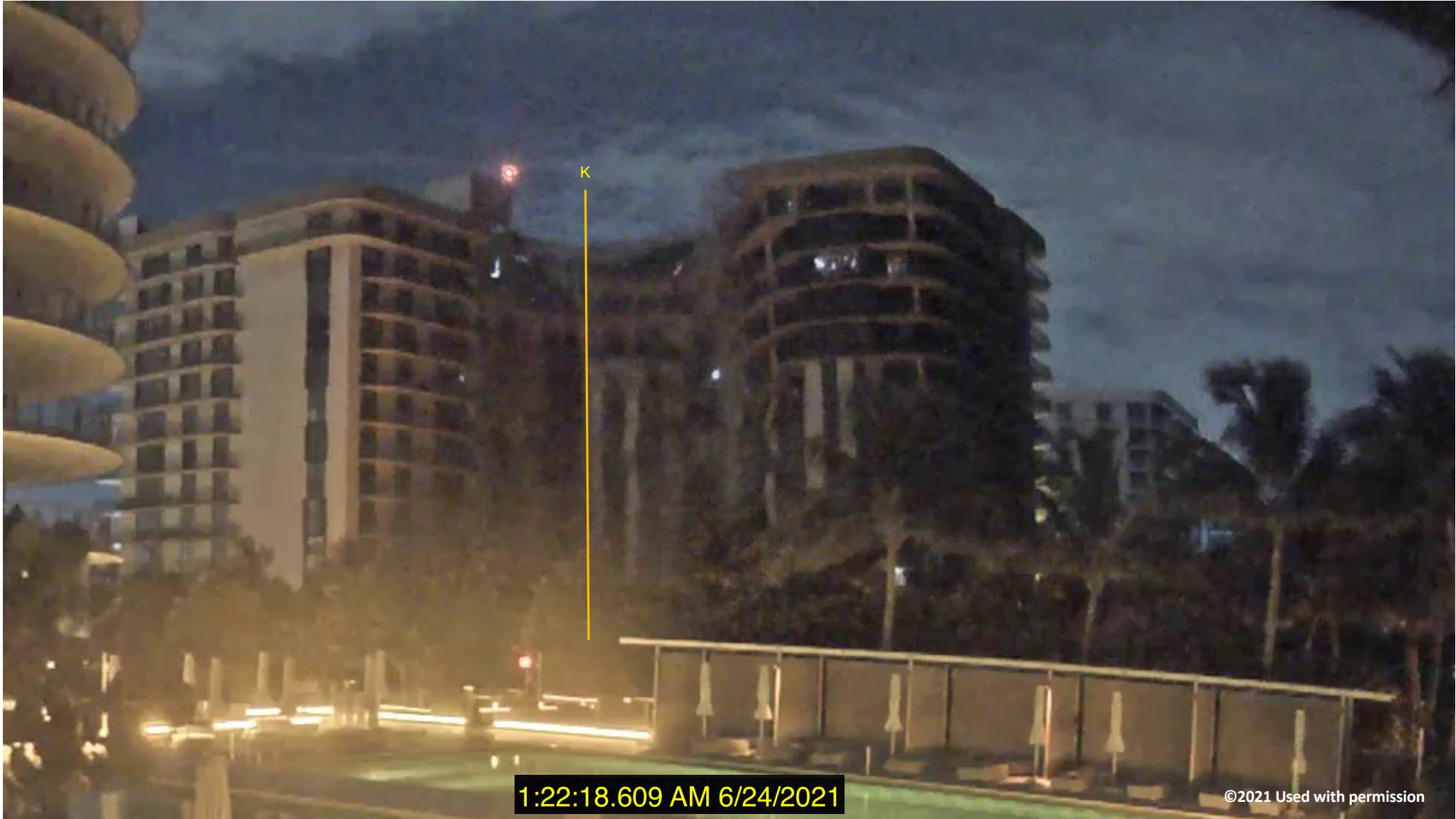
1:22:18.176 AM 6/24/2021

©2021 Used with permission



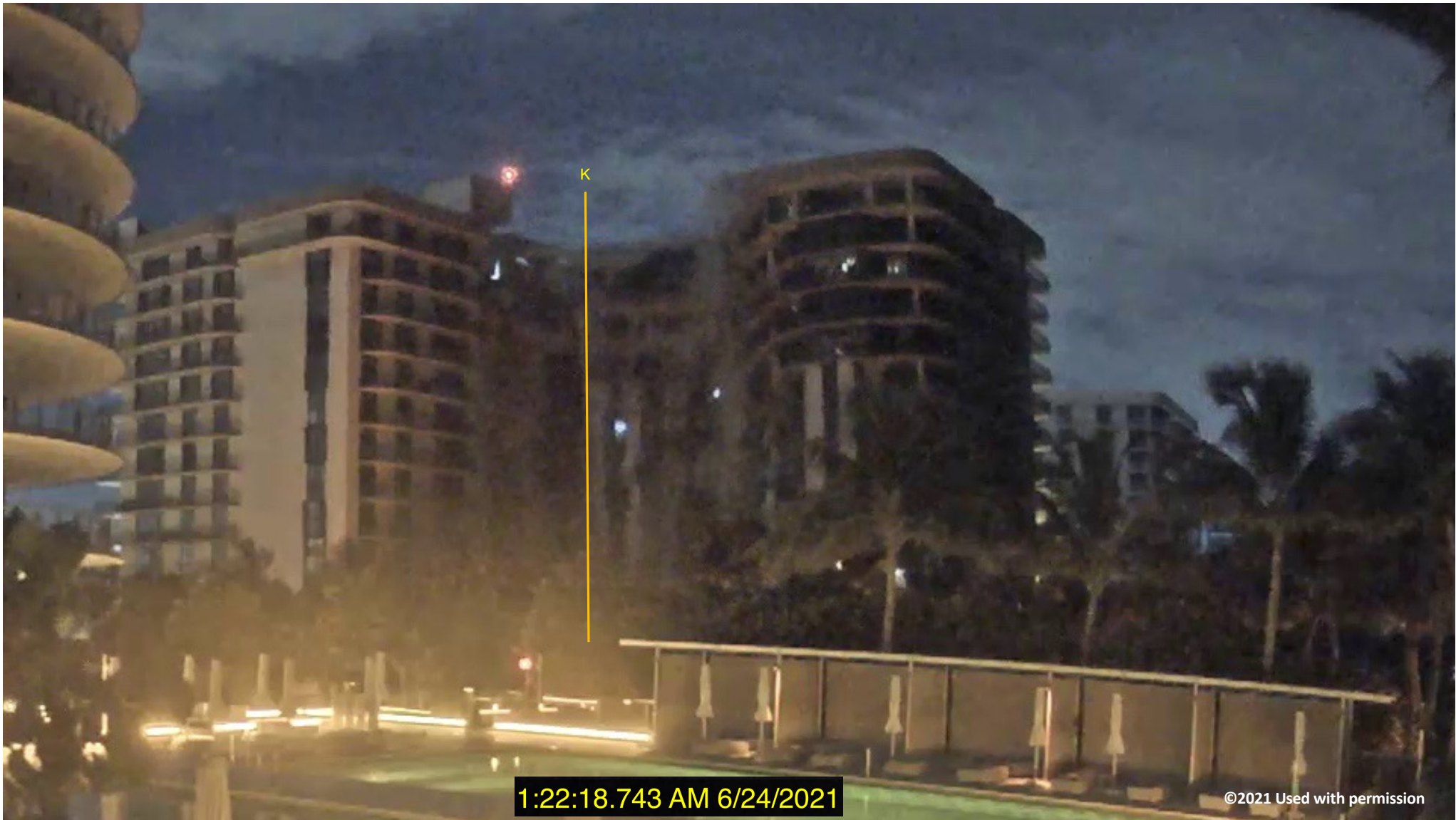
1:22:18.309 AM 6/24/2021

©2021 Used with permission



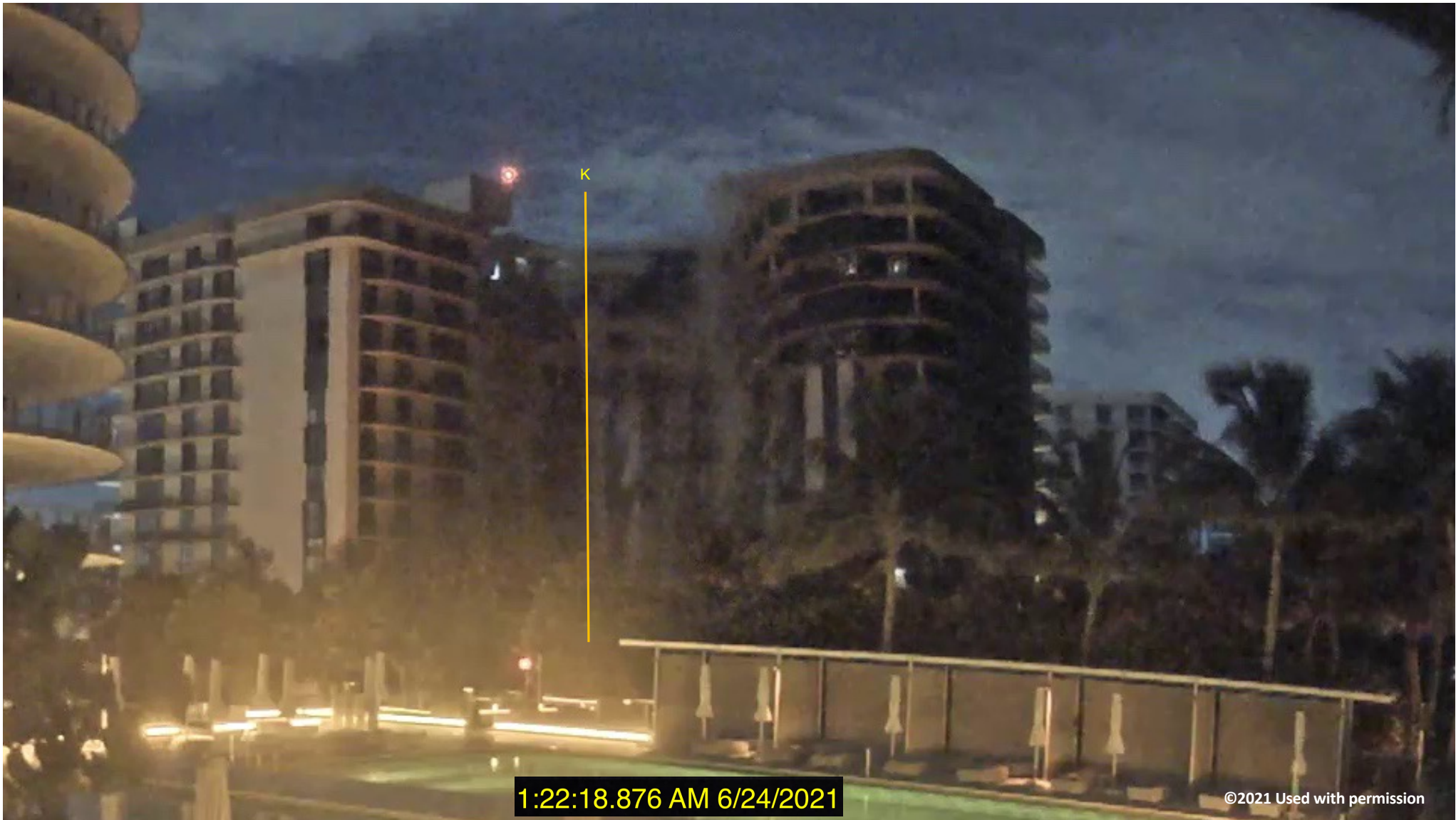
1:22:18.609 AM 6/24/2021

©2021 Used with permission



1:22:18.743 AM 6/24/2021

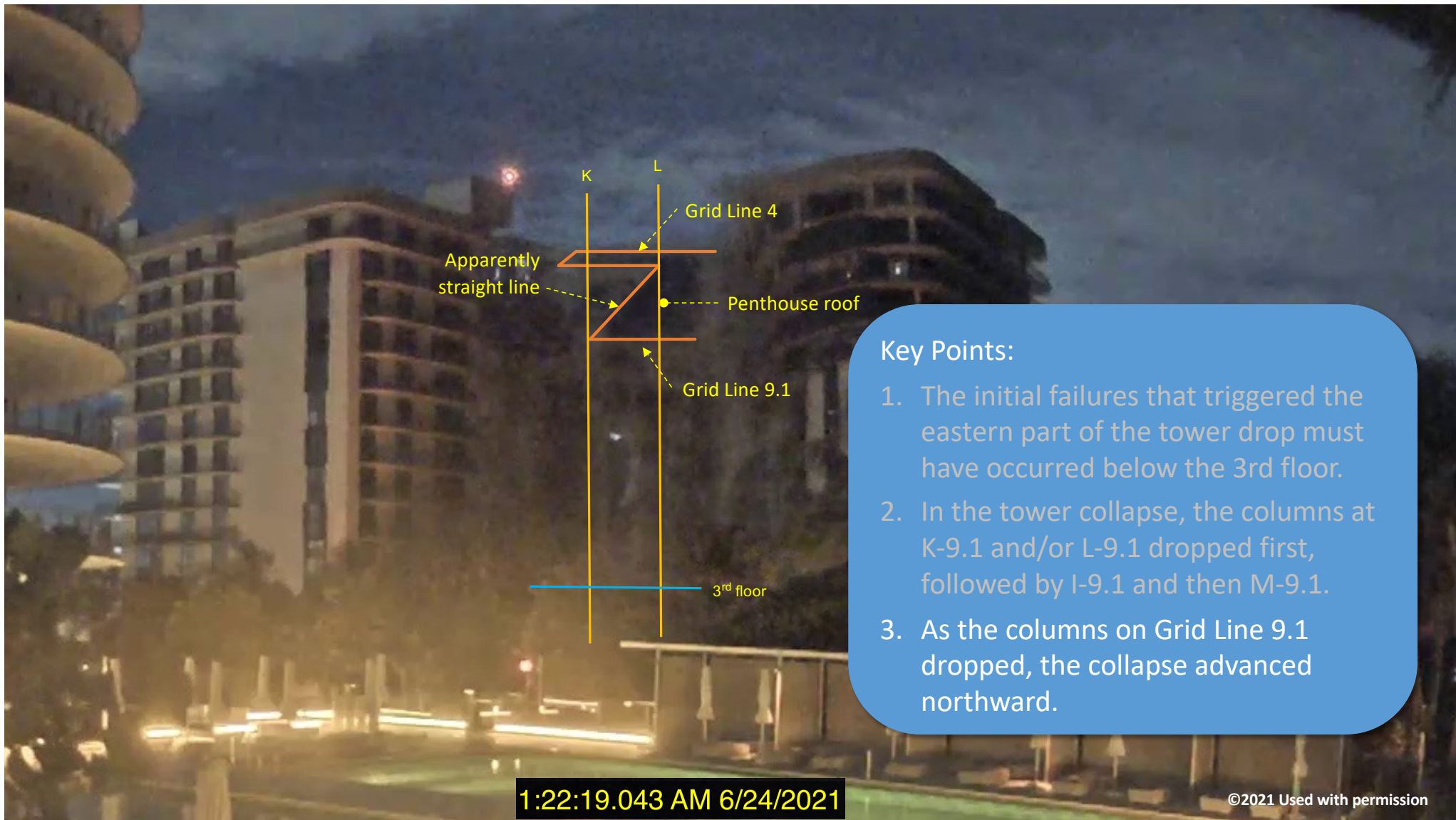
©2021 Used with permission



1:22:18.876 AM 6/24/2021

©2021 Used with permission





### Key Points:

1. The initial failures that triggered the eastern part of the tower drop must have occurred below the 3rd floor.
2. In the tower collapse, the columns at K-9.1 and/or L-9.1 dropped first, followed by I-9.1 and then M-9.1.
3. As the columns on Grid Line 9.1 dropped, the collapse advanced northward.

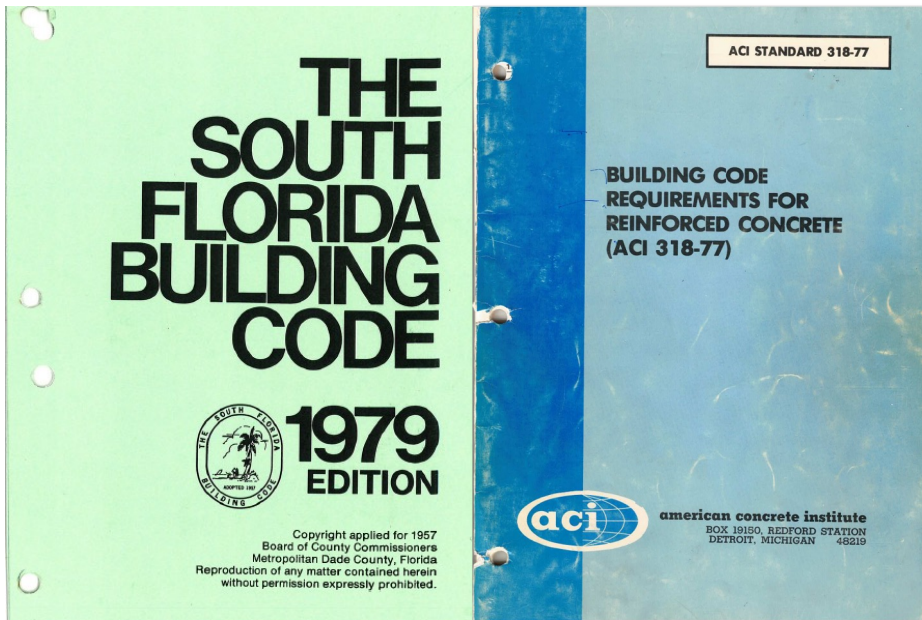
1:22:19.043 AM 6/24/2021

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04

As-built & Precollapse  
Conditions, Structural  
Tests, SSI Analyses

## Design Basis



Source: MDC

Source: ACI

Town of Surfside had hundreds of drawings from the permit application

SFBC: live and wind loads; testing of pile foundations

1977 ACI 318: Equivalent Frame Method for two-way slabs

*ASCE 7-22 specifies higher wind speeds and pressures*

*ACI 318-19 has many more requirements for exposure to chlorides, structural integrity, and minimum reinforcement over columns in two-way slabs*

# Permitting

- Permits were issued
- Partial 13<sup>th</sup> story added after initial submissions

**TOWN OF SURFSIDE**

Approved  
 Approved as noted  
 Disapproved

Inspector: *MJC*

**Fountain Plan**  
 1/4" = 1'-0"

**SOUTH ELEVATION @ 1/8" = 1'-0"**

**TOWN OF SURFSIDE**

APPROVED:

Bldg. Inspector: *MJC* Date: 11/17/80  
 Electrical Inspector: *[Signature]* Date: 11/17/80  
 Plumbing Inspector: *[Signature]* Date: 11/17/80  
 Mechanical Insp.: *[Signature]* Date:

Reviewed: *J. Roberts* Chairman  
 Planning & Zoning Board Date: 10/28-80

NEW PENTHOUSE ADDITION  
 WILLIAM M. FRIEDMAN  
 AND ASSOCIATES ARCHITECTS, INC.  
 75 G.L. HILL  
 188804  
 CHAMPLAIN TOWERS SOUTH  
 8731 COLLINGS BLVD  
 A. 705 UNIT CONDOMINIUM  
 DRAWN DATE: 8/8/79 [S.H.L.]  
 SCALE: 1/8" = 1'-0"  
 SHEET: 2 OF 6

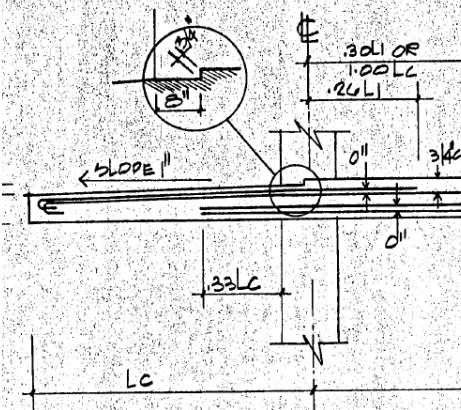
Source: Town of Surfside

# Design Detail Issues



## Congestion:

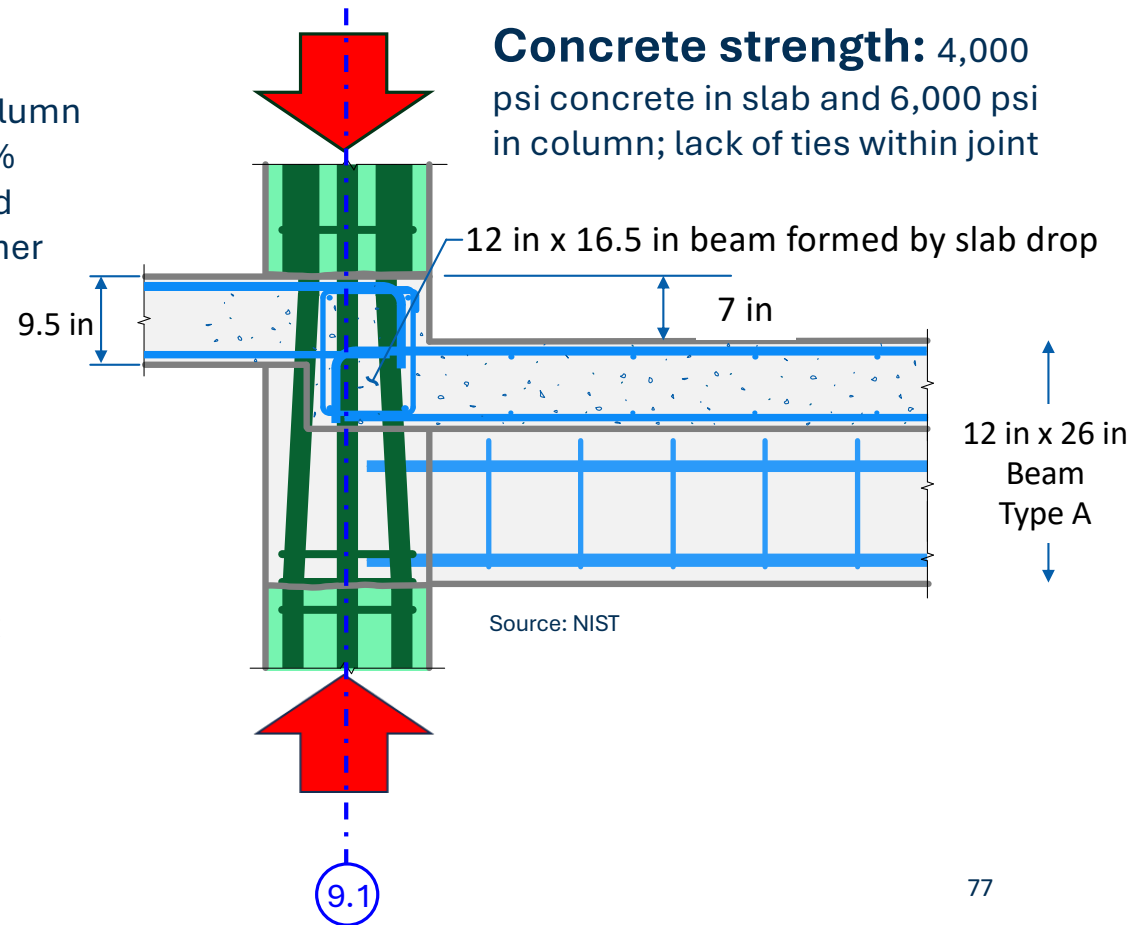
9.75 % vertical reinforcement in column at this splice, vs. 8 % maximum permitted (worst case at another column is 12.2 %)



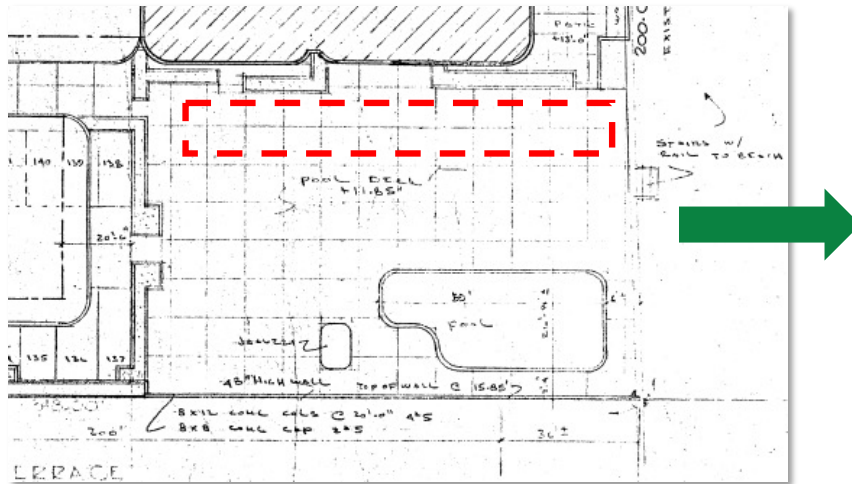
## Inadequate cover:

over reinforcement in balcony slabs exposed to weather

Source: Town of Surfside



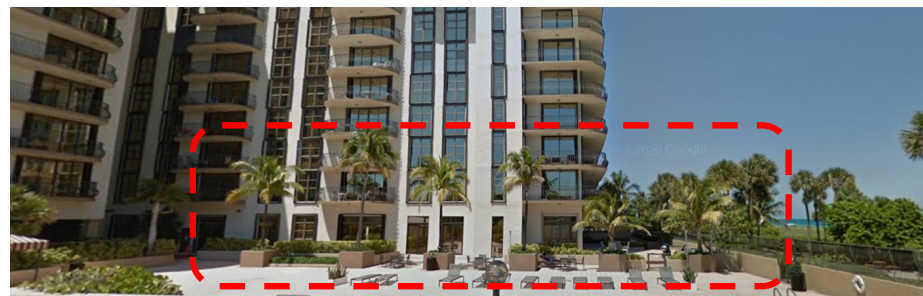
# Changes From Design Drawings



Original Architectural Drawing (from Town of Surfside)



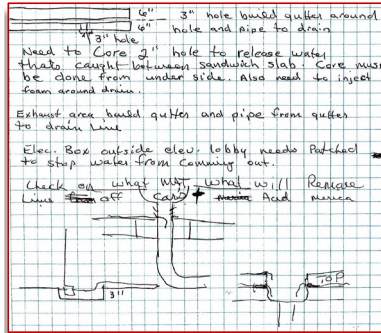
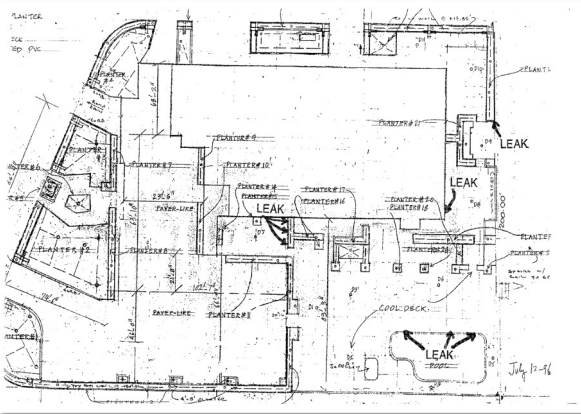
Source: Google Earth image captured November 2019, downloaded May 22, 2023



Source: Google Street View image captured March 2015, downloaded May 22, 2023

Palm trees  
were later  
removed

# As-Built Conditions



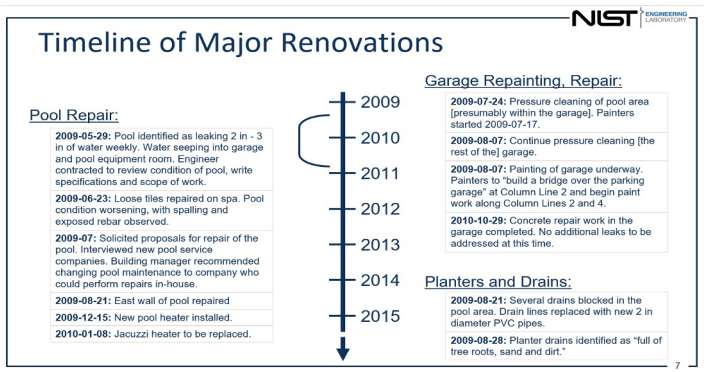
Source: CTS Receiver

Source: NIST



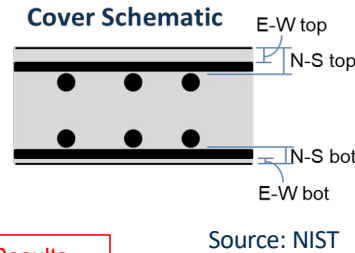
Tens of thousands of additional civil litigation files transferred to NIST since March 2024. Records (such as sample above) used by NIST to populate a timeline of pool deck renovations (sample portion below).

New digital evidence contributed to mapping of construction joints in pool deck.



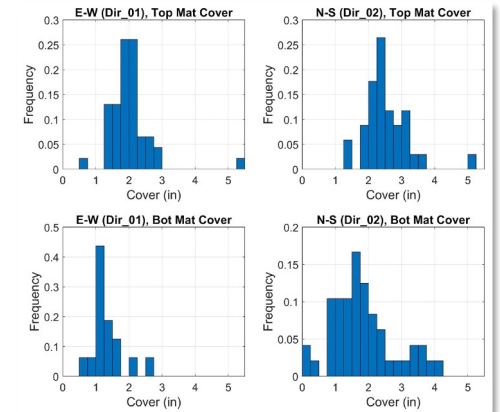
Adapted by NIST (Original Source: CTS Receiver)

Detailed measurements of specimens used to analyze compliance.

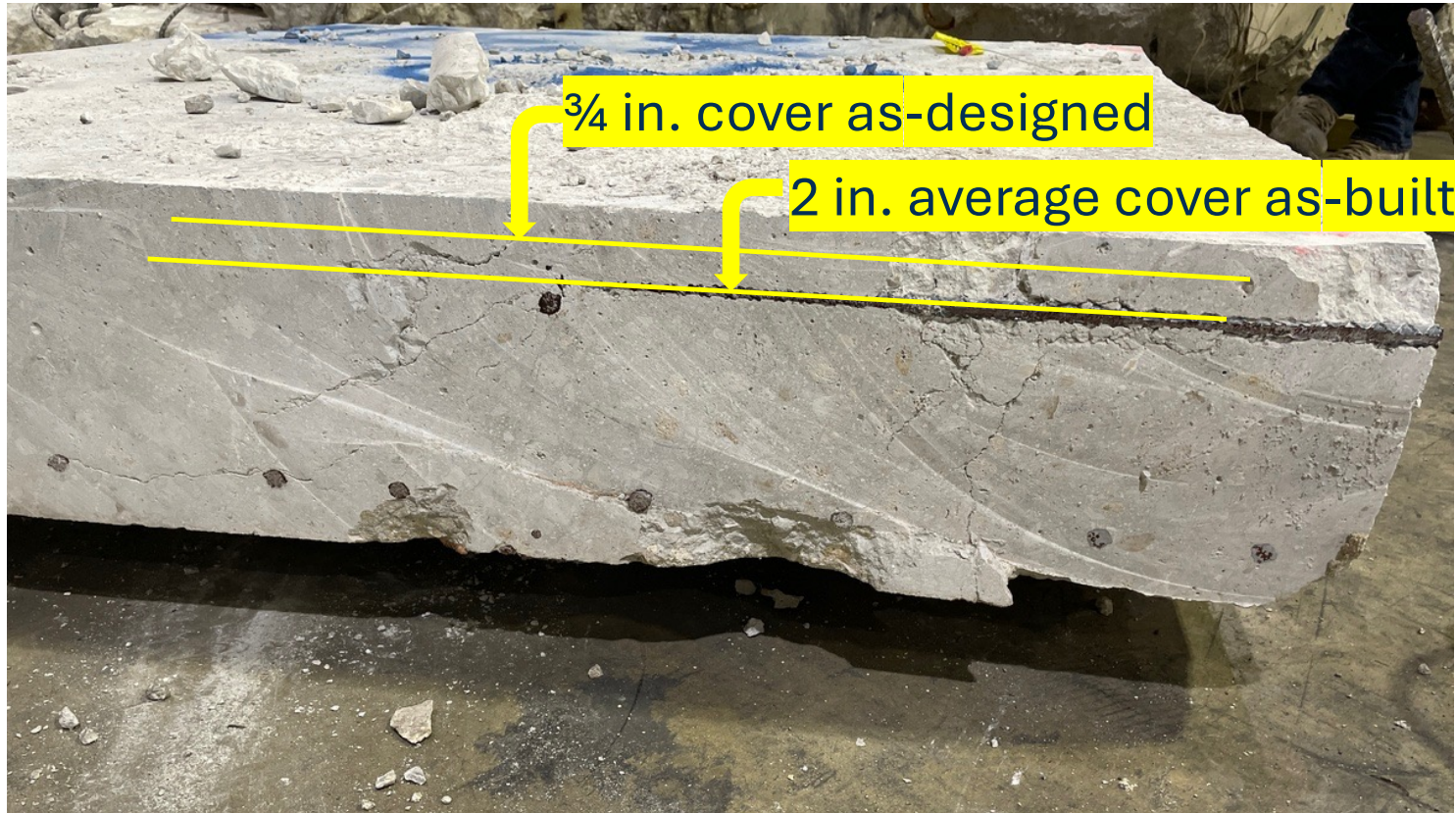


Source: NIST

Preliminary Analysis Results



## Closer View of Misplaced Top Reinforcement



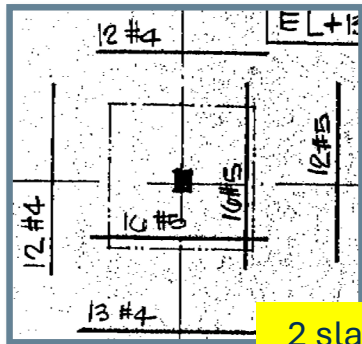
Source: NIST



# As-Built Conditions

Typically, fewer than the specified number of column strip top reinforcing bars are centered over the column in the pool deck slab.

⑤.- AT LEAST 25% OF ALL COLUMN STRIP BARS SHALL BE CENTERED OVER THE COLUMN.



Slab Top Reinforcement at Example Column Location

2 slab top reinforcement bars



Example Column Specimen

At this location, only 2 rather than 4 top bars were centered over the column in each direction.

2 slab top reinforcement bars

The measured spacing of the top reinforcing bars in the column strips of the pool deck slab specimens commonly ranges from about 20 % to 40 % wider than required by the structural design drawings, resulting in less reinforcing in the column strips than required by the design.



slab top reinforcement spacing

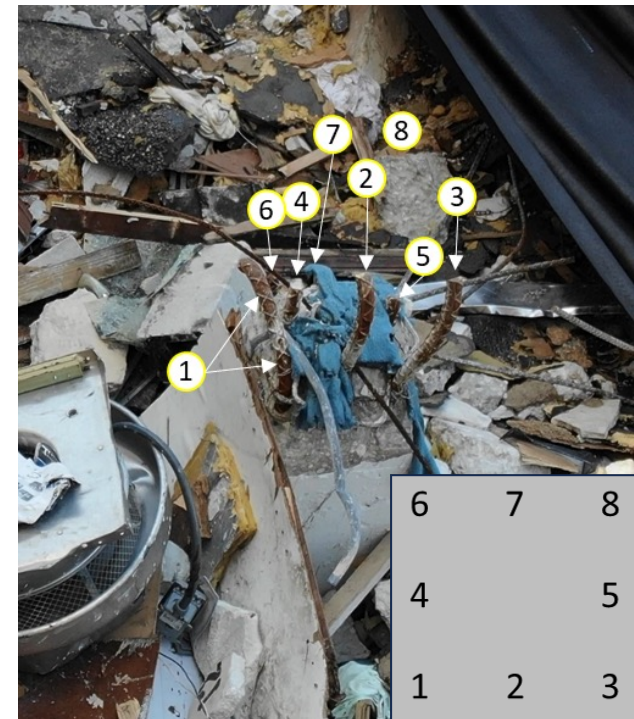
# As-Built Conditions



Source: NIST

## Position of reinforcing cage within columns:

Photos of top of basement column at Grid Line K on south face of tower: bars shifted to the north (excessive cover on near face, but ties against form on far side of column)



# As-Built Conditions



**Alignment of concrete:** Offset in column from story to story exceeds standard tolerances.

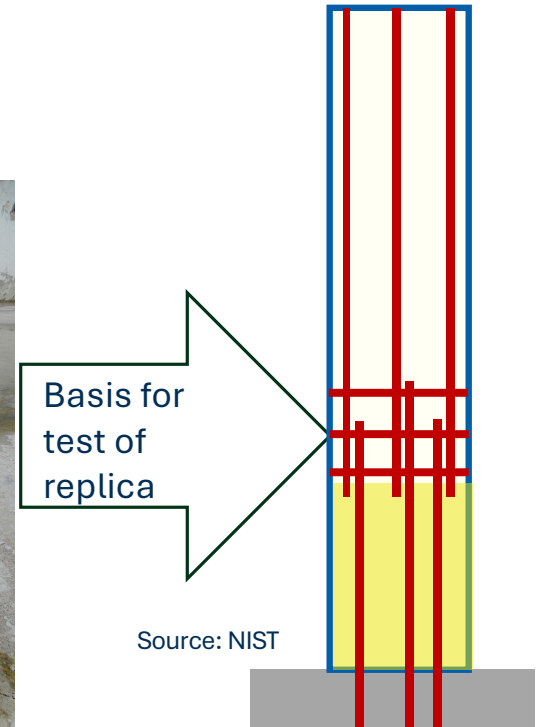
Source: NIST

Source: NIST



**Misplaced/short splice:**

Several columns found with longitudinal bars where the lap splice is shorter than specified.



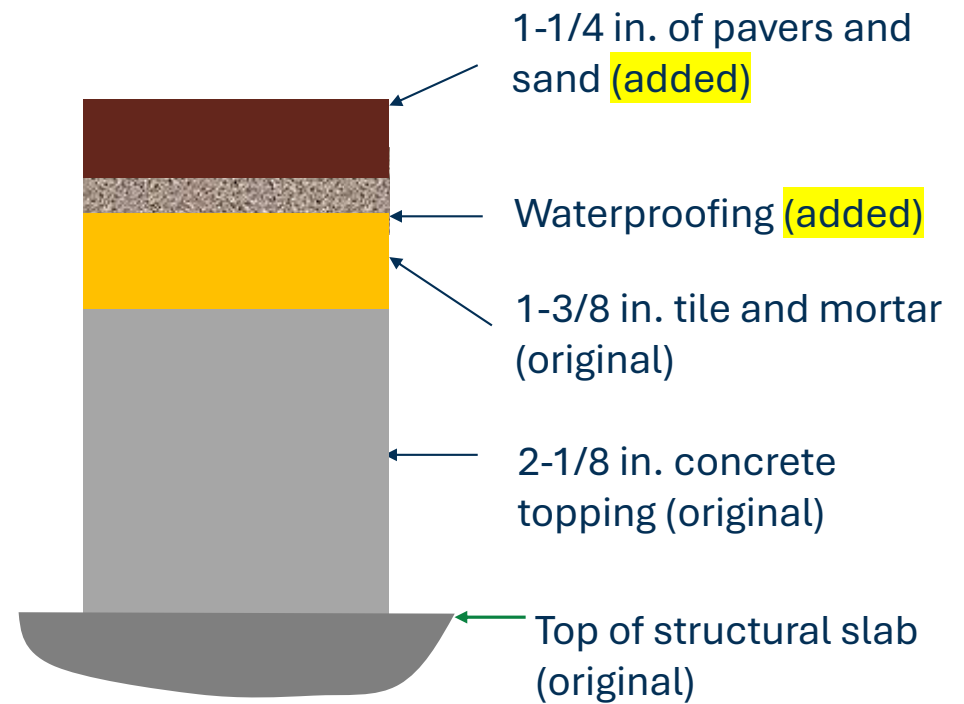
# 1995 Pool Deck Rehabilitation

## Examples of Additional Fill and Paving

9 in. **added** sand setting bed



Source: Structures Specialist  
from US&R Ohio Task Force 1

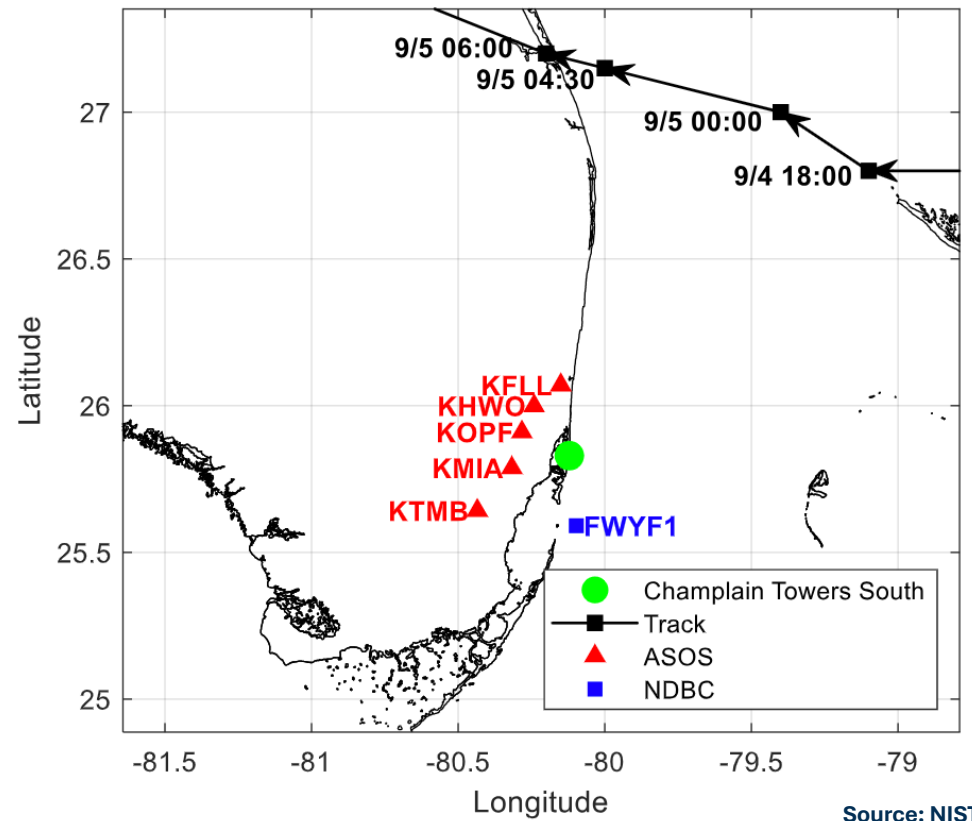


Schematic based on observations  
reported by Morabito Consultants

# Wind Exposure at the Site



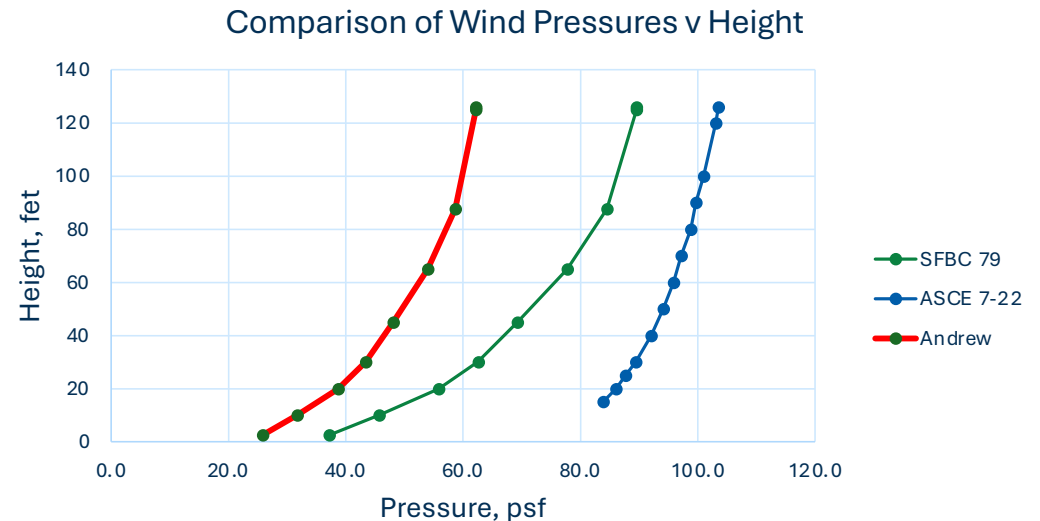
Source: NIST



Source: NIST

# Historical Winds

- NIST's wind contractor identified ten highest speed hurricanes over life of CTS
- NIST selected five for detailed analysis
- Contractor:
  - Developed wind field models for the five
  - Computed wind speed and direction at CTS for each



Source: NIST

Historical wind study found highest velocity winds at the building were from Hurricane Andrew, 1992. The pressures were less than required by the 1979 SFBC or by ASCE 7-22.

PRELIMINARY ANALYSIS RESULTS

# Evidence of Corrosion

Corrosion of Pool Deck Hooked Column Reinforcement Bars that Were Embedded into Pool Deck Slab



Enlargement of Column Top Showing Hooked Reinforcement Bars with Corrosion



Pool Deck Column Post-Collapse

Source: NIST



Same Column in Primary Evidence Facility

# Aged Material Properties



Left: some of the specimens in the warehouse

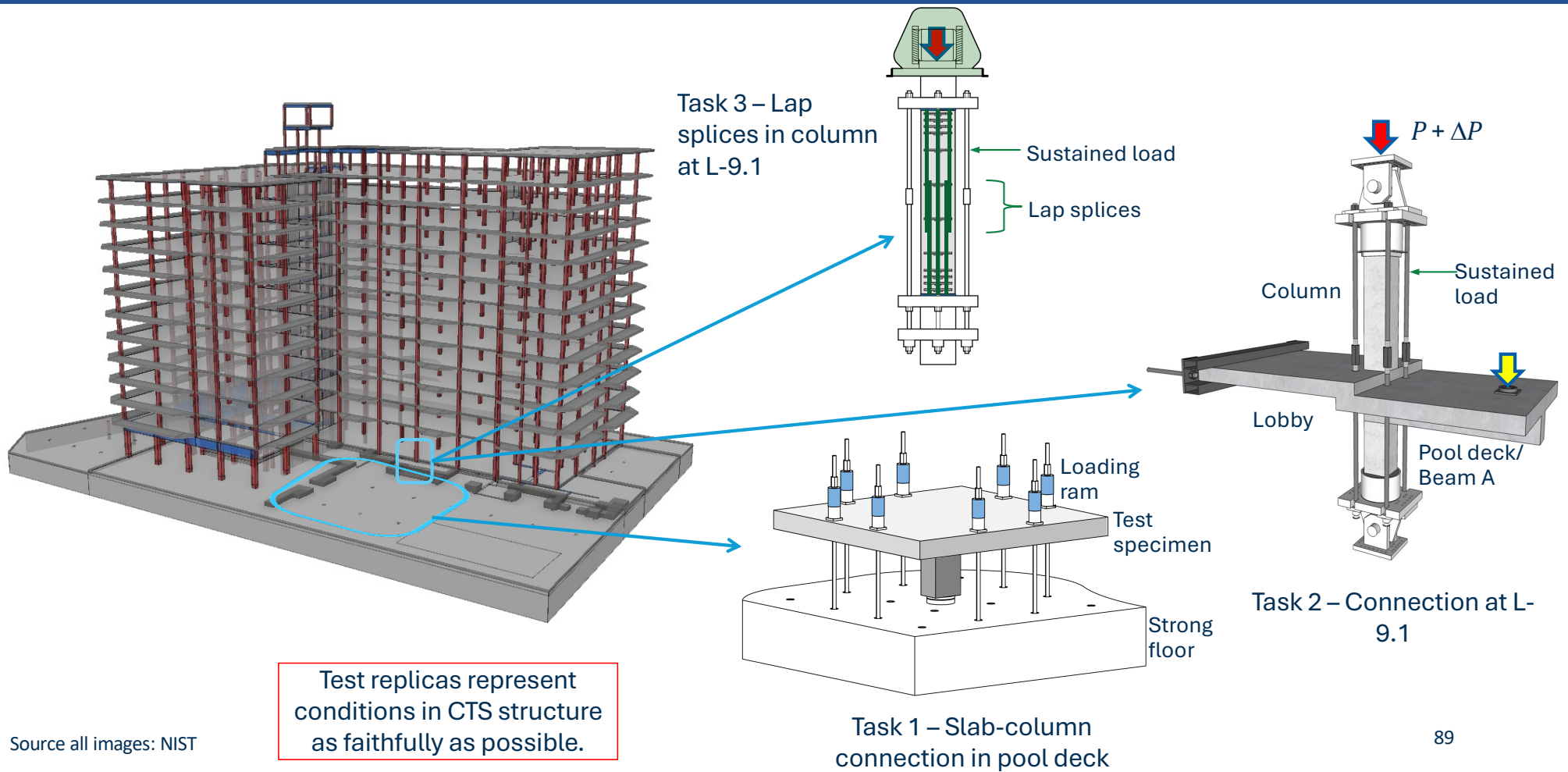
Right: removing samples of concrete for laboratory testing



Source: NIST



# Structural Laboratory Tests



Source all images: NIST

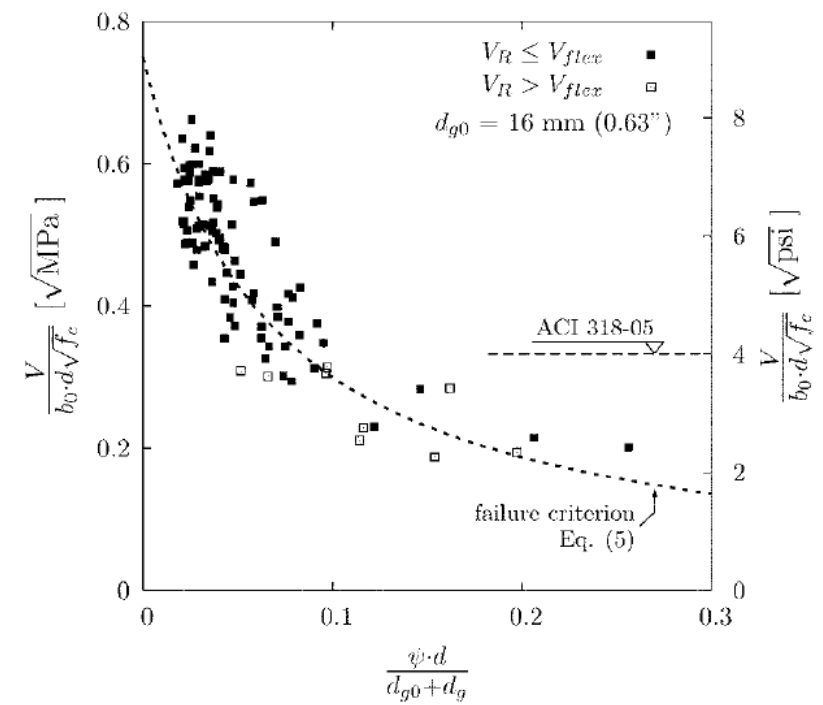
# Slab-Column Connection Tests

Test in progress



Source: NIST

Critical shear crack theory

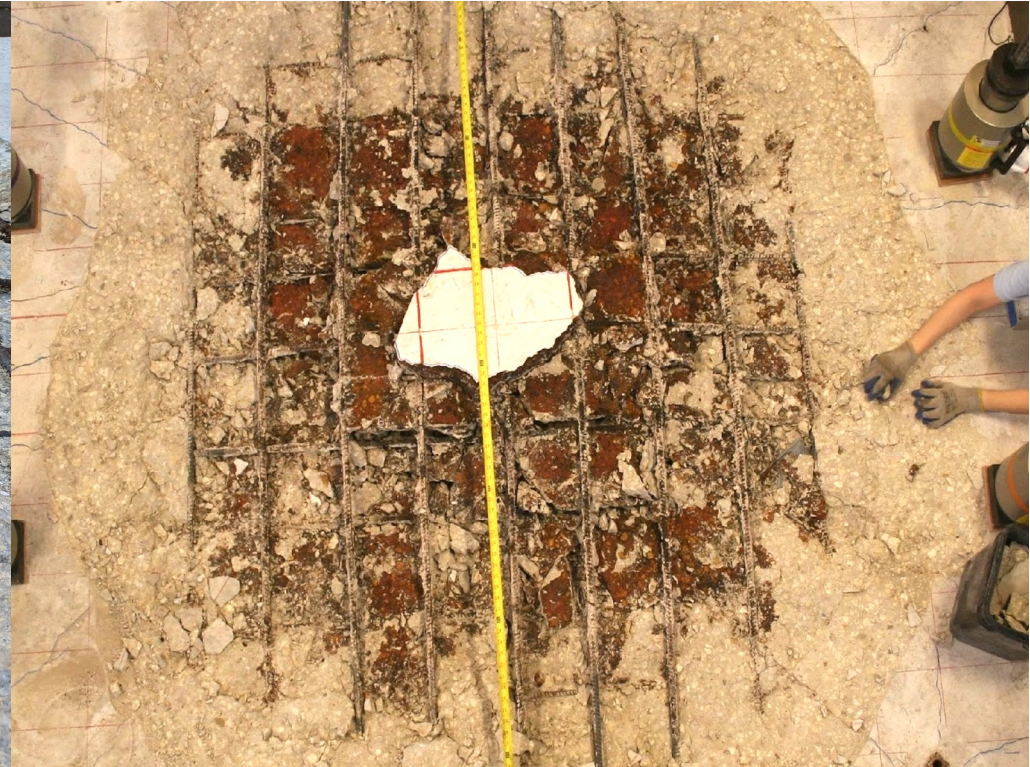


Source: ACI

# Slab-Column Connection Tests

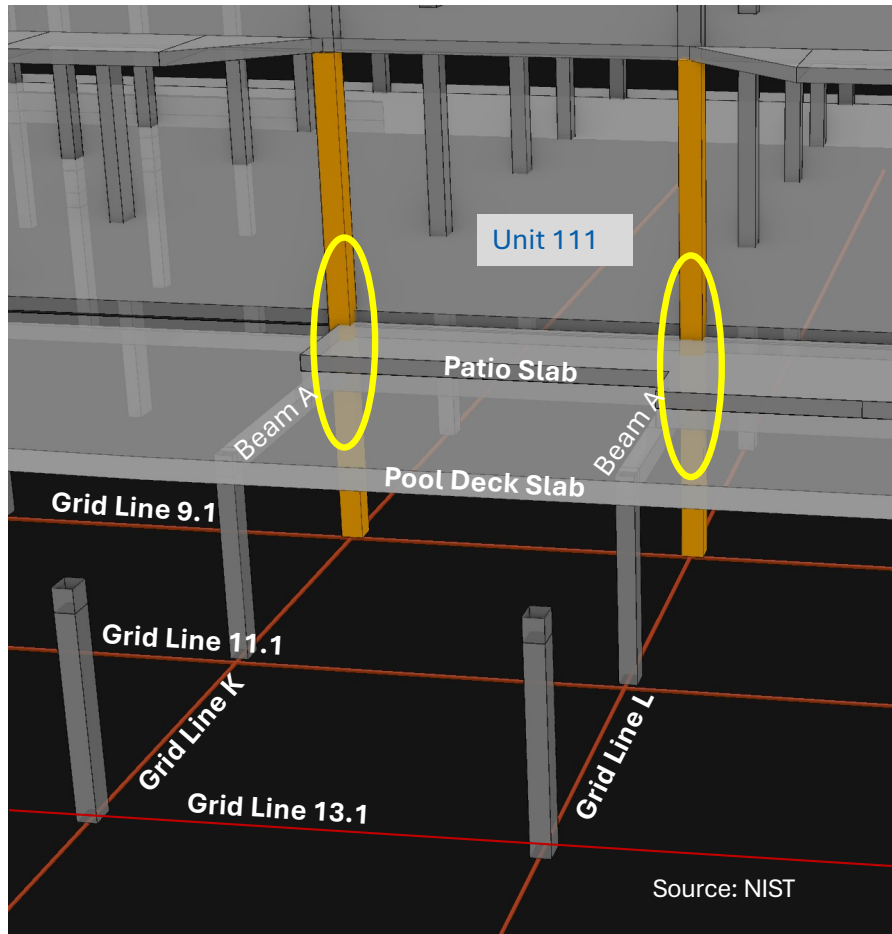


Post-test, uncorroded



Post-Test, corroded

# Columns Along South Edge of Tower



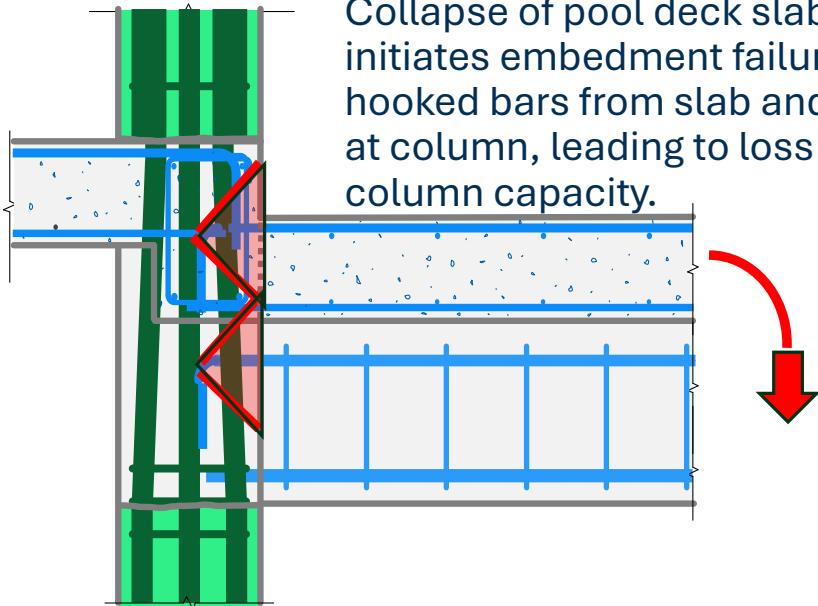
## Description of Structure

### Structural Columns at Grid Lines K and L along the South Edge of the Tower

- Unit 111's patio is 7 in down from interior floor.
- The pool deck is another 11 in down from the patio. (Shown terminating at Grid 11.1 for clarity of structure below)
- The slab drop beams run along each step.
- Additional beams (Type A) extend from south face of tower to the next row of columns under the pool deck.

# Columns Along South Edge of Tower

Collapse of pool deck slab initiates embedment failure of hooked bars from slab and beam at column, leading to loss of column capacity.



## Description of Failure Progression Hypothesis

### Important Issues

- Position of ends of hooked bars
- Strength of concrete
- Position of column reinforcement (vertical and ties)
- Lack of column ties in joint
- Corrosion of reinforcement

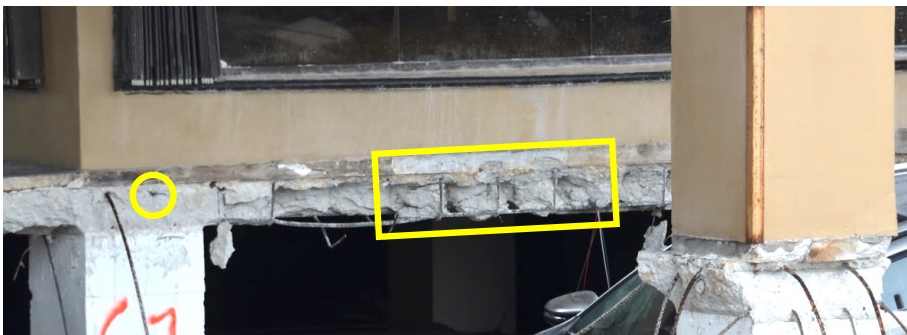
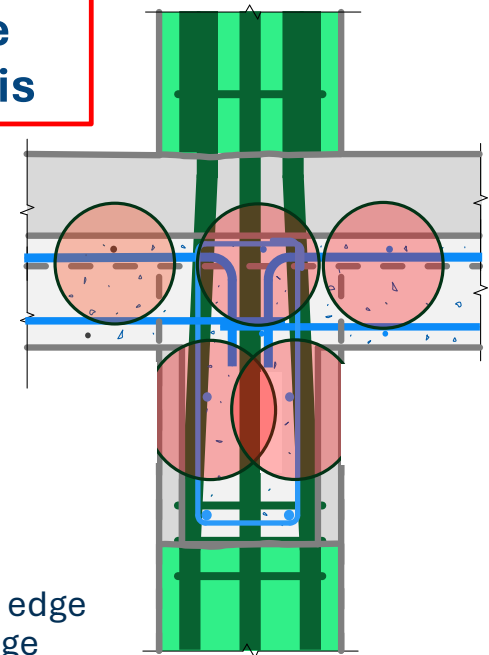
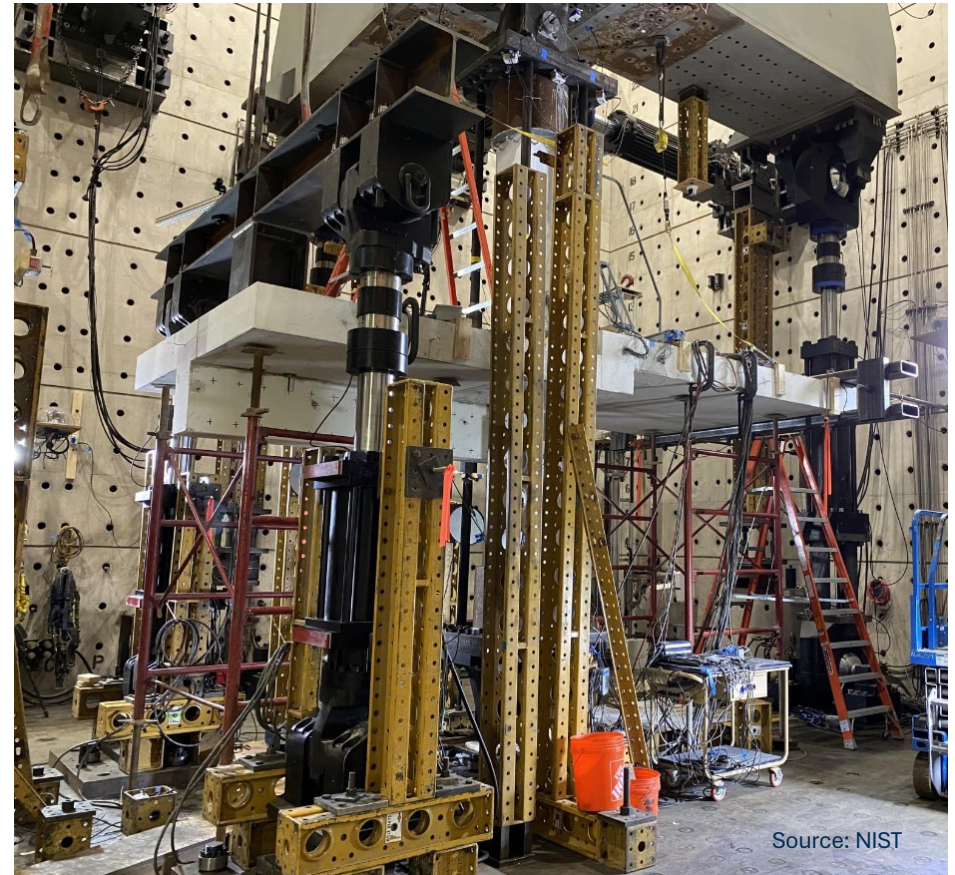
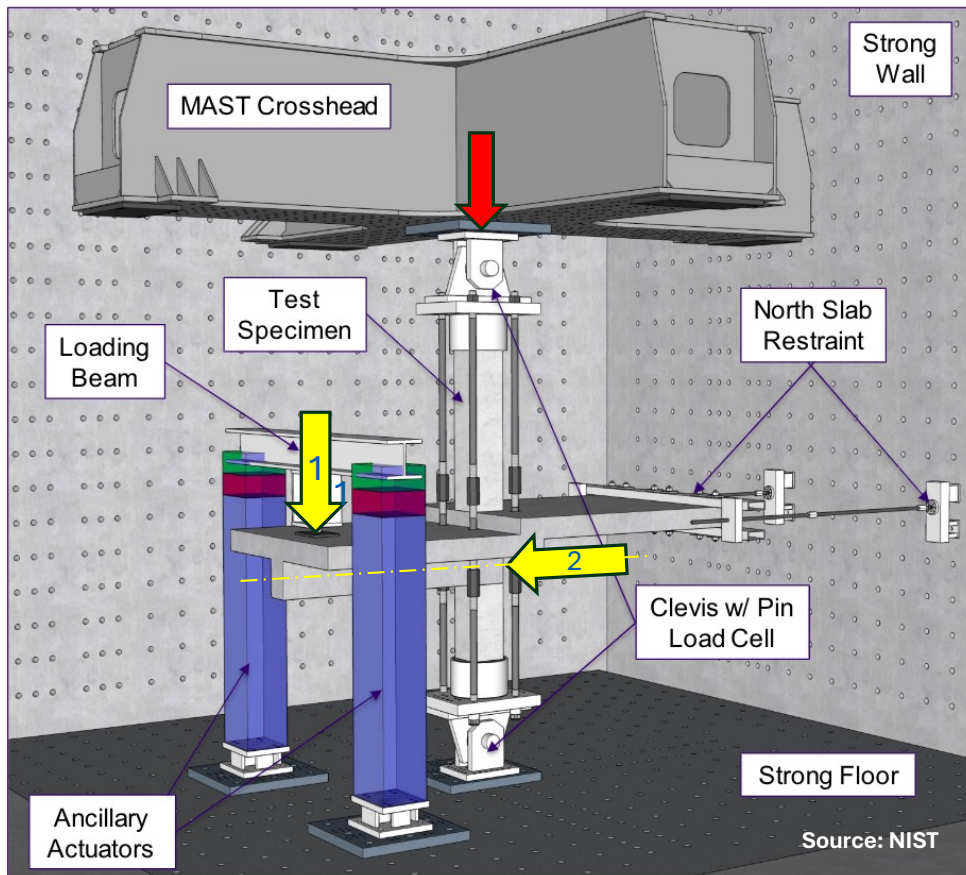


Image (at left) of similar condition at edge of portion that did not collapse. Image analysis of area in yellow box led to the profile (below) of concrete left after bars pulled out.

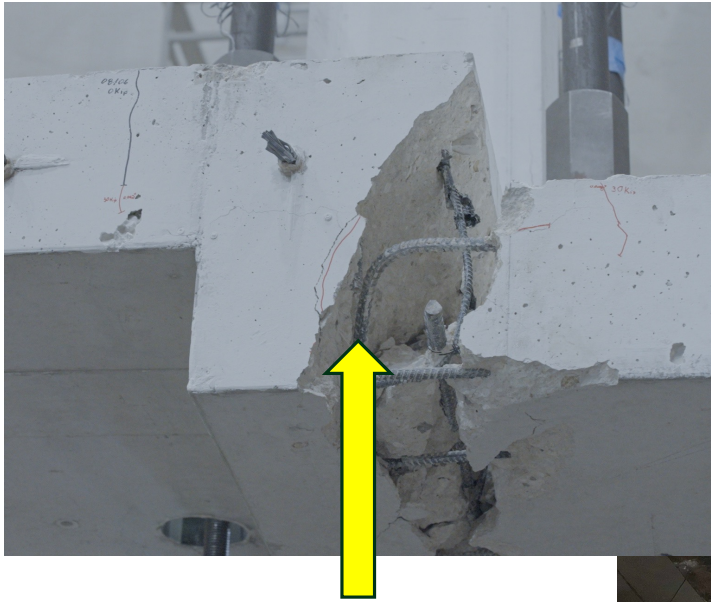


Source all images:  
NIST

# Structural Laboratory Tests – Task 2

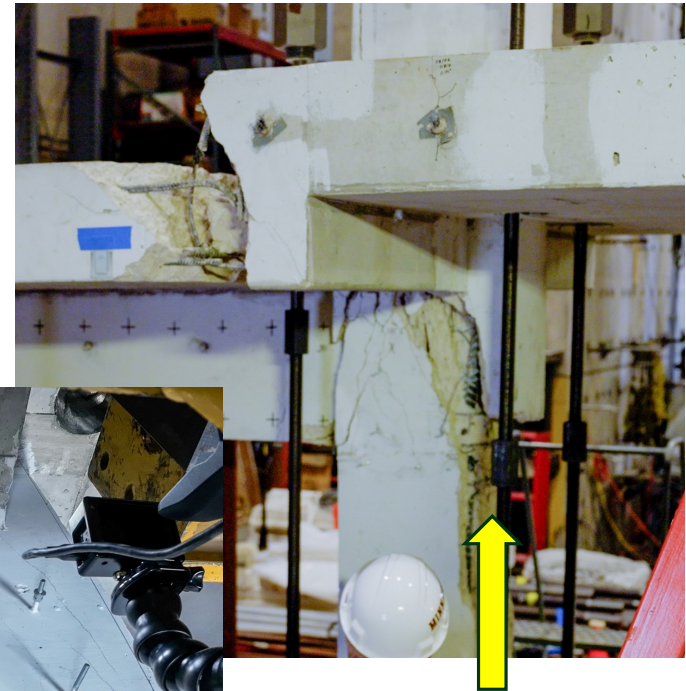


# Joint Damage in Tests



Hooked #5 top bars in lower slab broke out of drop beam

Test #2: column bars at south face lose cover and buckle



Test #1, column bars at north face lose cover and buckle

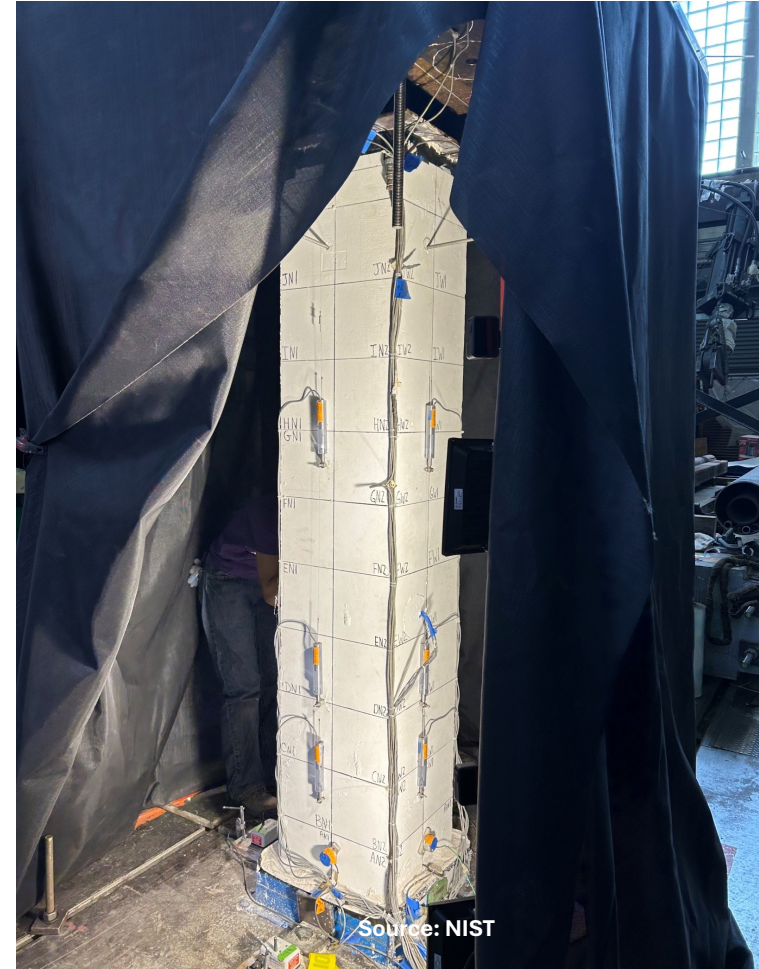
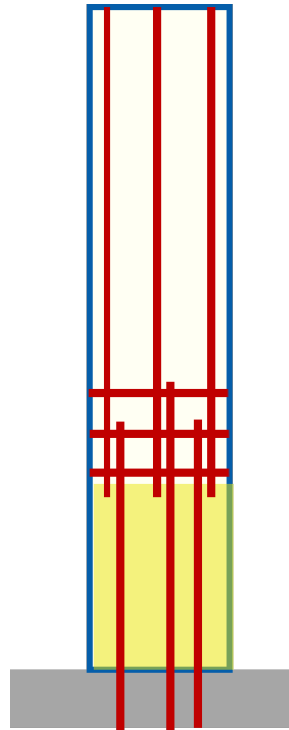


Source all images: NIST

# August 2024: Structural Tests at UW



Tests to evaluate effect of short lap splices found on column bars





# Short Lap Splice Test of Replica Column



Left: specimen with short lap splice and sustained load

Source all images: NIST

Below: close up of same specimen



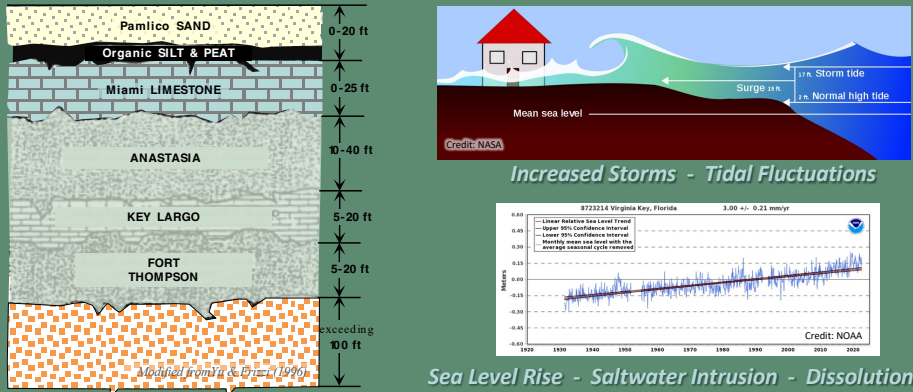
Right: specimen with no lap splices



# Geotechnical and Historical Data

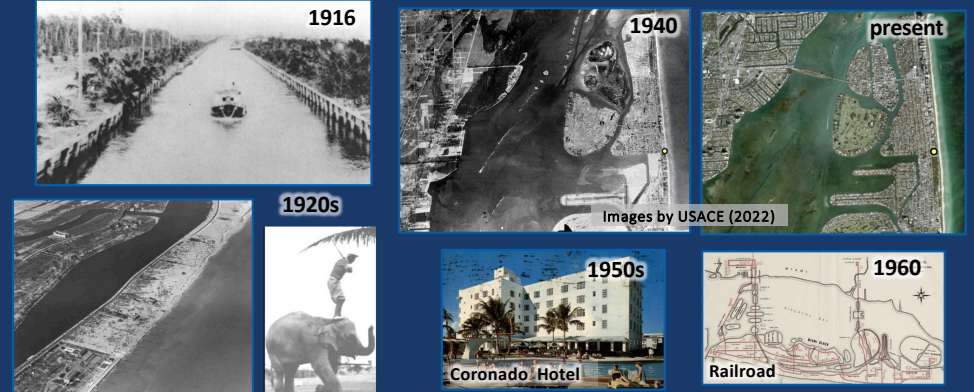
## Regional Geologic Setting and Hazards

1



## Beachfront Development History

2



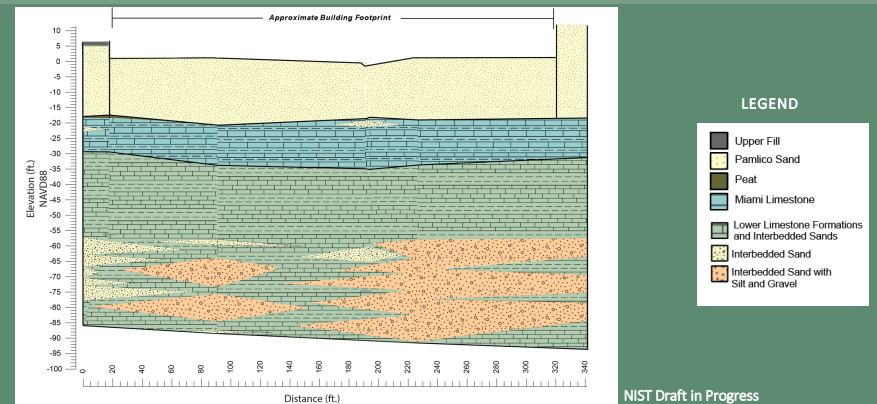
## CTS Site History

3

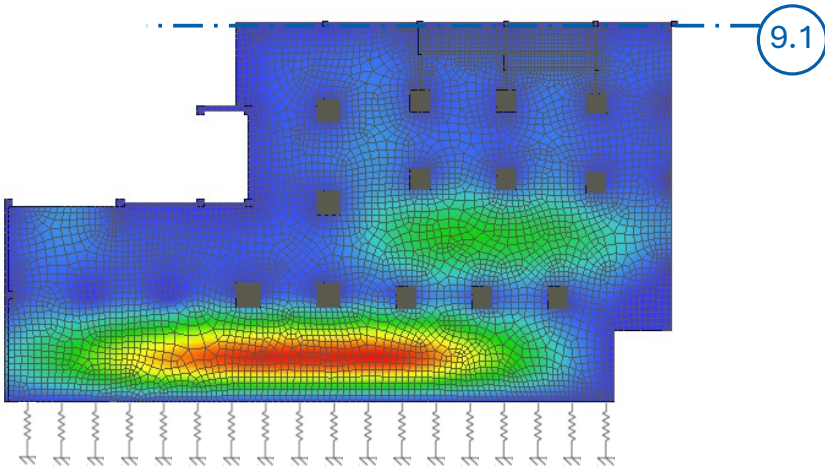


## Site-Specific Subsurface Investigations and Evaluations

4



# Soil-Structure Interaction Analysis



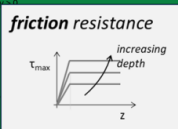
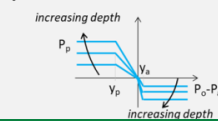
- As two-way slab cracks and the bars yield, the slab extends in-plane.
- Restraint offered by south basement wall and the soil outside the wall can influence the failure load in the slab.
- Careful modeling of the wall, the original sheet piling, and the soil outside the wall provides data to add nonlinear springs to the analytical model of the slab.

## South Wall Lateral Behavior Analyses

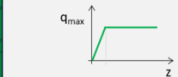
Soil resistance



passive and active earth pressures



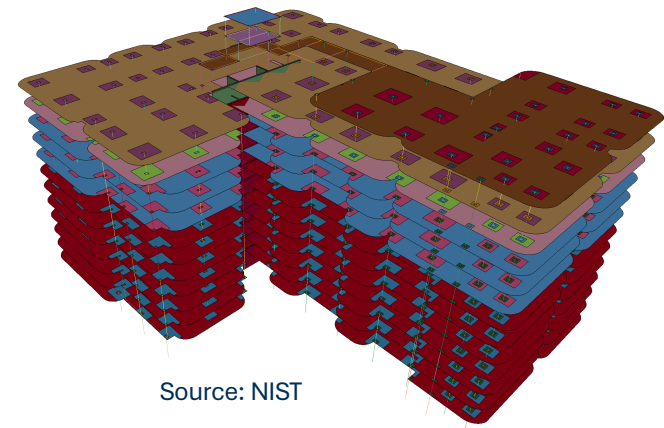
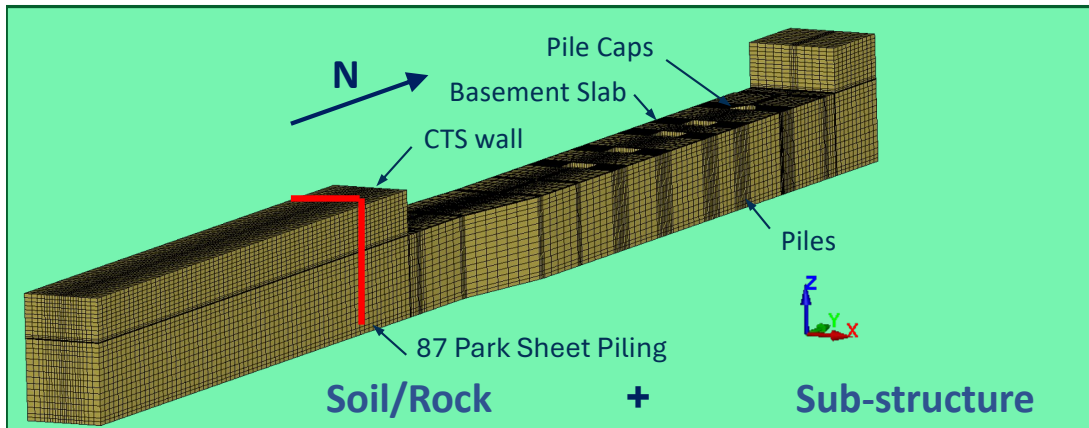
end bearing resistance



# Soil-Structure Interaction Analysis

Effects of the construction of 87 Park:

- Vibrations from driving of sheet piling
- Subsequent excavation



3D “Slice” model created to study vibrations from sheet pile installation and excavation at 87 Park site. Motions from that model used as base motion input for analysis of building response.

Source: NIST using LS-DYNA software

05

Materials Testing, Code Checks,  
Computer Simulations, Status of  
Failure Analysis

# Materials Testing and Evaluation

# Materials Testing and Evaluation

## Goals

- Characterize the mechanical and other physical properties of the of concrete
  - Concrete mixes, concrete elements, casting times and placements, and exposure conditions
- Establish existing concrete mixtures through petrography and chemistry
  - Analyze maturity and degradation / Create surrogate concrete
- Characterize the mechanical properties of the reinforcing steel
  - Identify the extent, severity, effect, and causes of corrosion



## Materials Sampling

- Organize parts of structure into **representative groups**
- Extract sufficient samples in each representative group to recognize the variability of material properties in that group and tolerance for error

$$n = (1.96\sigma/E)^2$$



# Materials Testing and Evaluation



## 594 Tests Completed for Mechanical Properties of Concrete

Compression and  
modulus of  
elasticity tests

Splitting tensile  
tests



Source for all images: NIST



# Materials Testing and Evaluation

## 183 Tests Completed for Mechanical Properties of Reinforcing Steel



Measurement of degree of corrosion of steel reinforcing bars  
(assisted by NIST Statistical Engineering Division)

# Materials Testing and Evaluation

## Concrete Mixture Design

### Determine aggregate proportions and size:

- Air voids, mortar, and aggregate fractions determined by “point count” method and petrography
- Aggregate size determined by statistics-based visual comparison

### Determine mixture proportions for surrogate concrete:

- Coarse aggregate and water fraction held constant for workability
- Water-to-cement ratio (w/c) varied to control strength
- Results show that ~ 4000 psi can be achieved at high w/c
- Implications for structure service life and durability



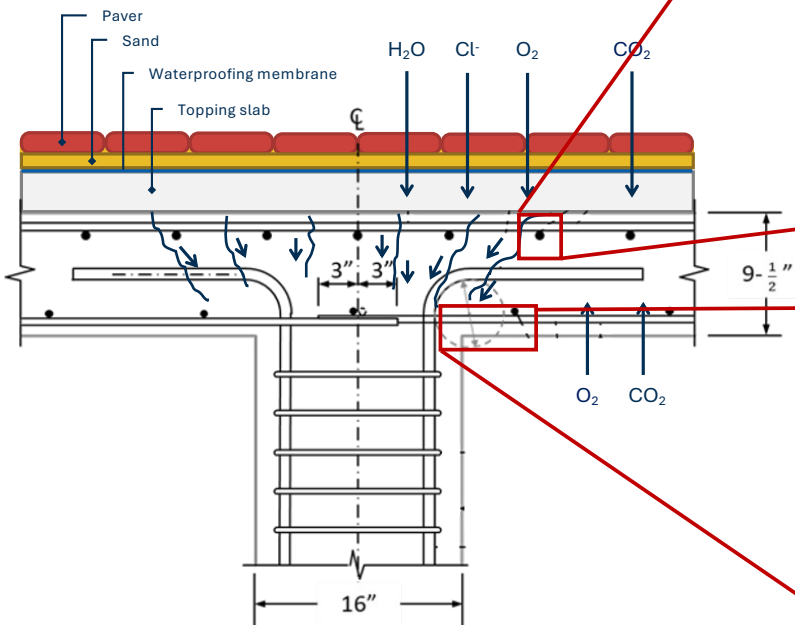
PRELIMINARY ANALYSIS RESULTS

Source for all images: NIST

# Materials Testing and Evaluation

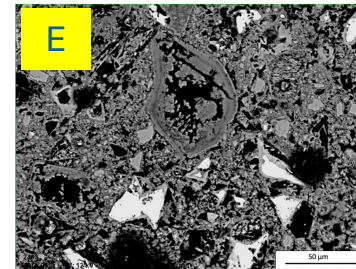
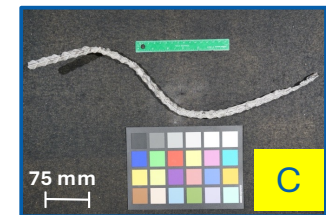
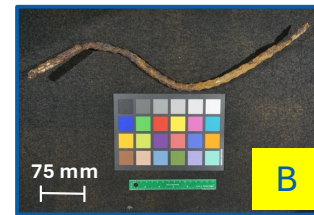
## Degradation Mechanisms

1. Cracking creates a “highway” for  $\text{Cl}^-$ ,  $\text{H}_2\text{O}$ , and  $\text{CO}_2$  to reach reinforcing bar causing corrosion
2. Concrete microstructure altered by the environment, changing mechanical properties



### 1. Reinforcing bar corrosion

- Corroded reinforcement in slab (A)
- Reinforcement before (B) and after (C) cleaning



### 2. Concrete degradation

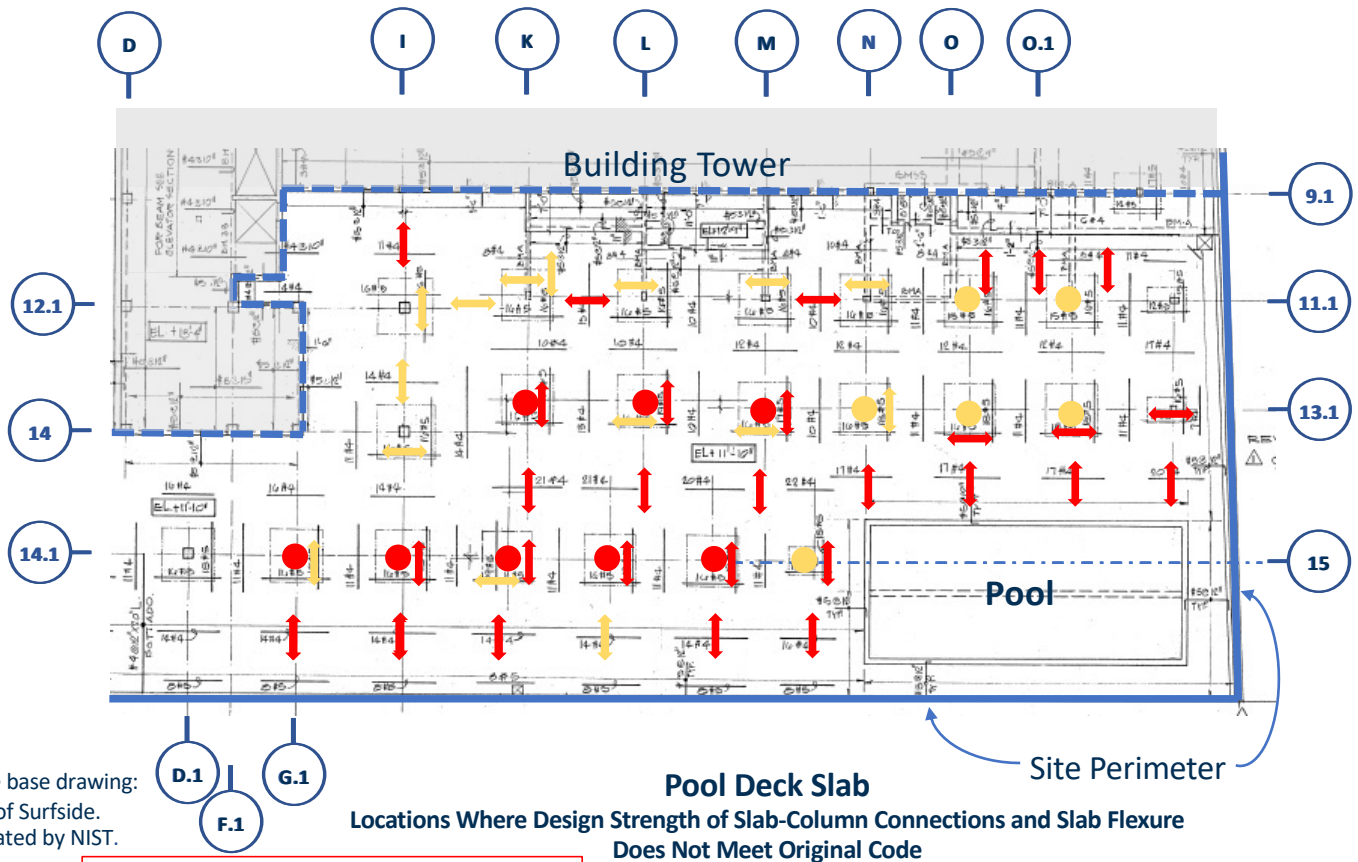
- Specimen extracted from slab near column (D)
- Microstructure characterized by high porosity (black regions at E)

Source for all images: NIST

PRELIMINARY ANALYSIS RESULTS

# Structural Code Checks

# Structural Code Checks



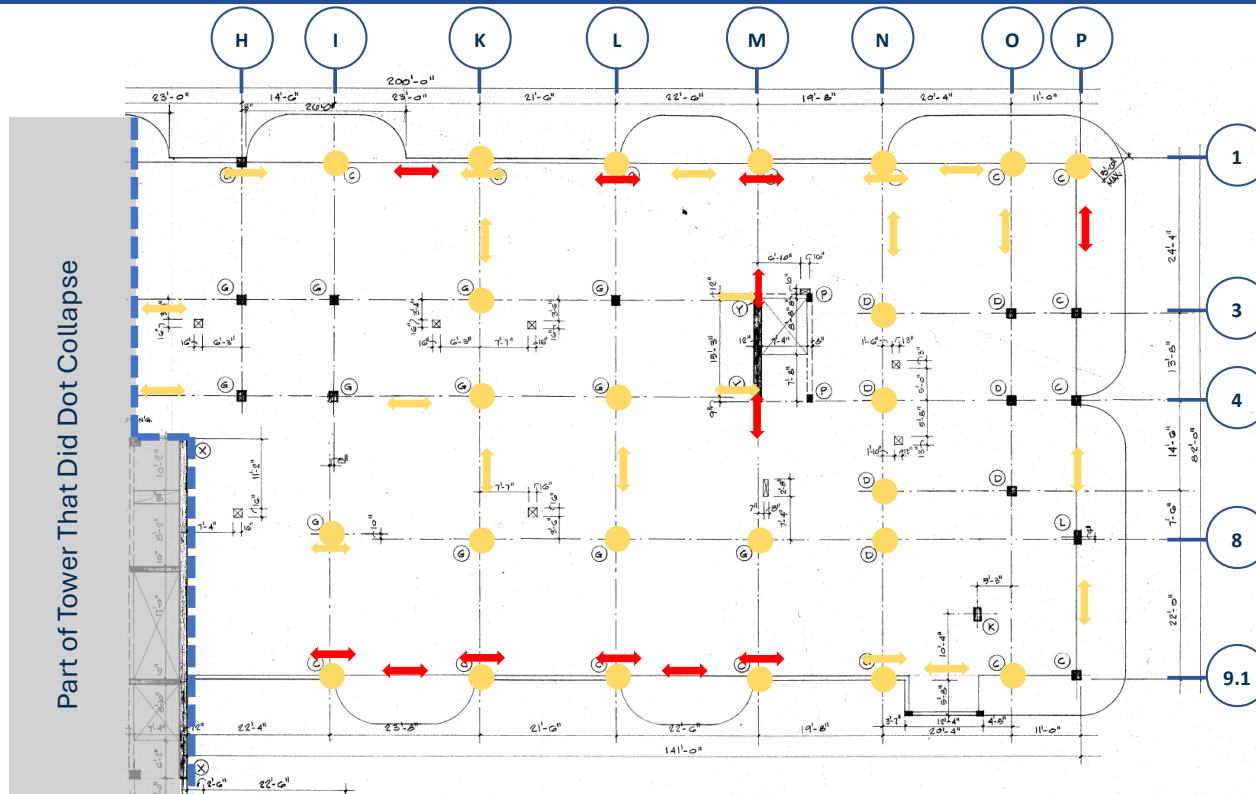
Source base drawing:  
Town of Surfside.  
Annotated by NIST.

**PRELIMINARY ANALYSIS RESULTS**

## Legend

Degree of Understrength	Location of Understrength	
	slab-column connections	slab flexure
severe	●	↔
moderate	●	↔

# Structural Code Checks



Source base drawing:  
Town of Surfside.  
Annotated by NIST.

**Third Floor Slab**  
Locations Where Design Strength of Slab-Column Connections and Slab Flexure Does Not Meet Original Code (DCR > 1.0)

**PRELIMINARY ANALYSIS RESULTS**

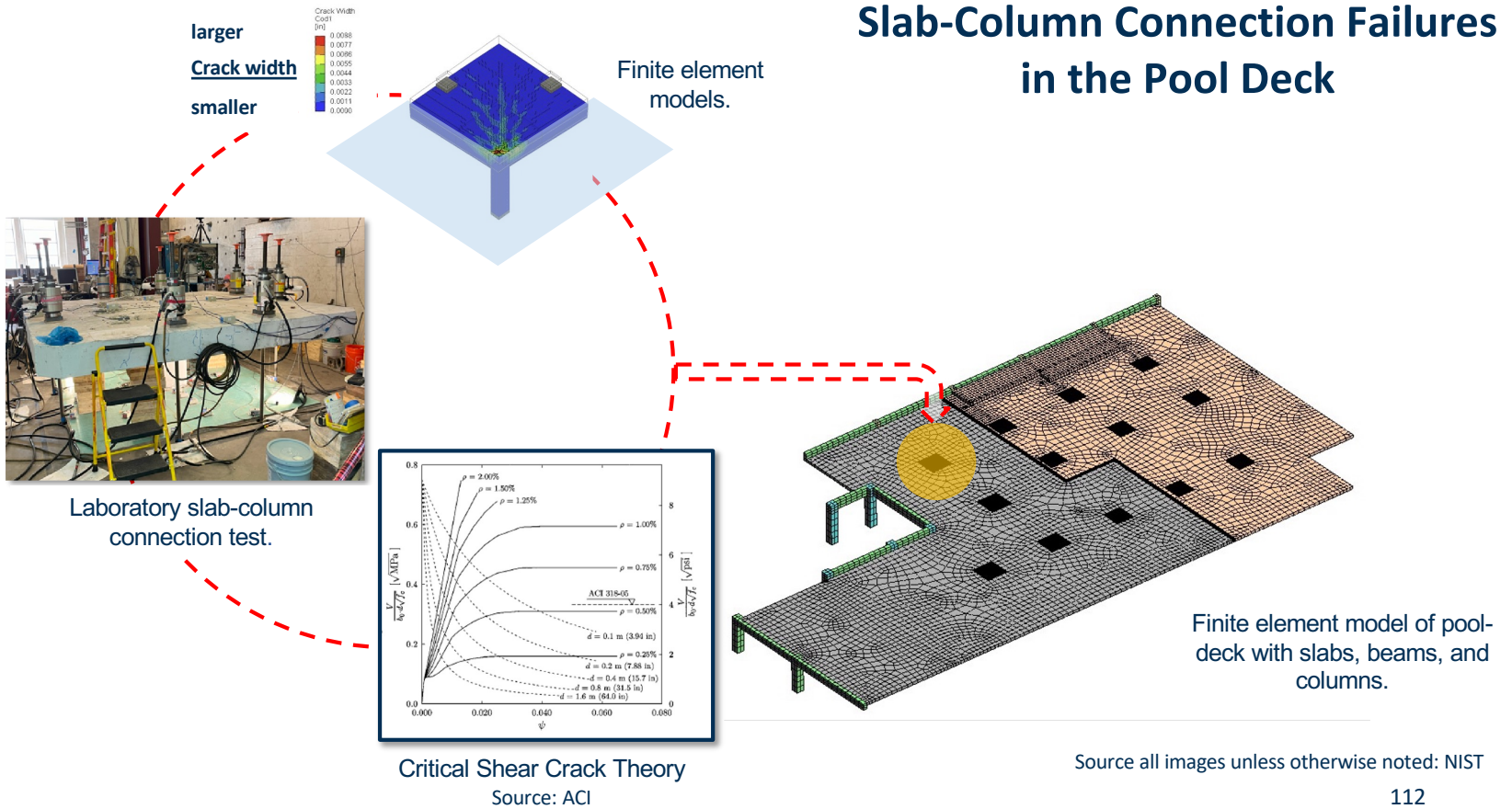
## Legend

Degree of Understrength	Location of Understrength	
	slab-column connections	slab flexure
severe	●	↔
moderate	●	↔

# Structural Failure Modeling

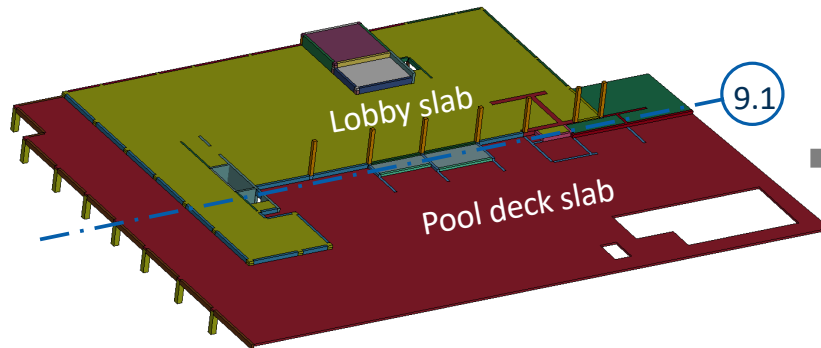
# Structural Failure Modeling

## Slab-Column Connection Failures in the Pool Deck



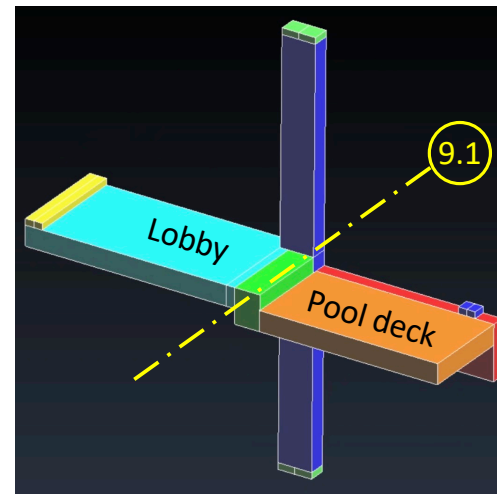


# Structural Failure Modeling



1 Pool Deck and First Floor Slab Models (ATENA & LS-DYNA)

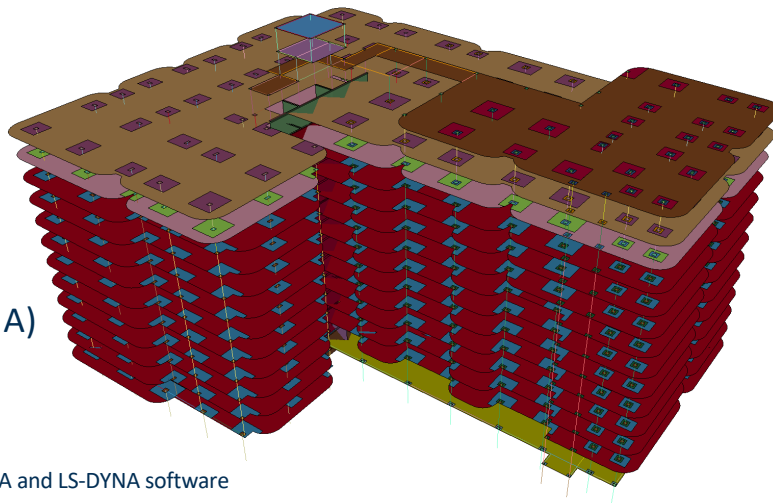
## Global and Progression Models



2 Slab-Beam-Column Connection Model (ATENA)

3 Global Tower Model (LS-DYNA)

3



Source: NIST using ATENA and LS-DYNA software

# Preliminary Pool Deck Evaluation

# Preliminary Pool Deck Evaluation

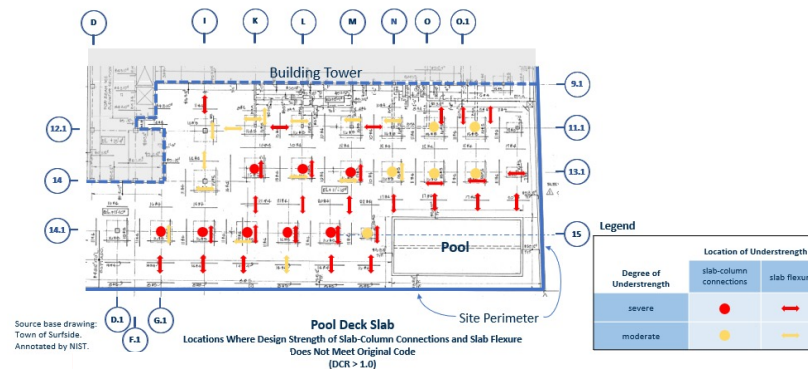
## Factors that Led to Critically Low Margins of Safety in the Pool Deck

PRELIMINARY ANALYSIS RESULTS

# Preliminary Pool Deck Evaluation

## Factors that Led to Critically Low Margins of Safety in the Pool Deck

**Design Understrength**  
(largest, pervasive)



PRELIMINARY ANALYSIS RESULTS

# Preliminary Pool Deck Evaluation

## Factors that Led to Critically Low Margins of Safety in the Pool Deck

**Design Understrength**  
(largest, pervasive)

**Misplaced Slab Reinforcement**  
(pervasive)



Top Reinforcement in Pool Deck Lower than Design Requirements    Placement of Top Bars in Column Strips

# Preliminary Pool Deck Evaluation

## Factors that Led to Critically Low Margins of Safety in the Pool Deck

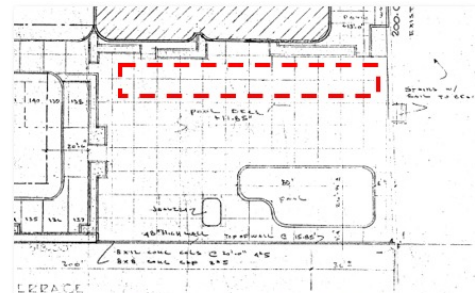
**Design Understrength**  
(largest, pervasive)

**Misplaced Slab Reinforcement**  
(pervasive)

**Heavier, More Extensive Planters**  
(near north side of pool deck)



Source: Google Earth image captured November 2019, downloaded May 22, 2023



Source base drawing: Town of Surfside  
Annotated by NIST

# Preliminary Pool Deck Evaluation

## Factors that Led to Critically Low Margins of Safety in the Pool Deck

**Design Understrength**  
(largest, pervasive)

**Misplaced Slab Reinforcement**  
(pervasive)

**Heavier, More Extensive Planters**  
(near north side of pool deck)

**Added Fill and Paving**  
(variable)



# Preliminary Pool Deck Evaluation

## Factors that Led to Critically Low Margins of Safety in the Pool Deck

**Design Understrength**  
(largest, pervasive)

**Misplaced Slab Reinforcement**  
(pervasive)

**Heavier, More Extensive Planters**  
(near north side of pool deck)

**Added Fill and Paving**  
(variable)

**Corrosion of Slab Reinforcement**  
(variable)





# Preliminary Pool Deck Evaluation

## Factors that Led to Critically Low Margins of Safety in the Pool Deck


**Design Understrength**  
(largest, pervasive)

**Misplaced Slab Reinforcement**  
(pervasive)

**Heavier, More Extensive Planters**  
(near north side of pool deck)

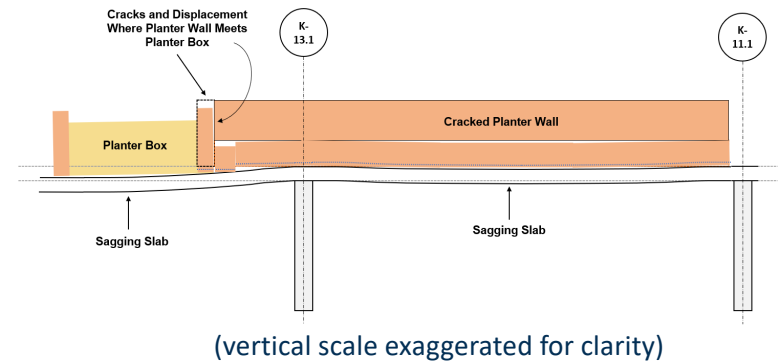
**Added Fill and Paving**  
(variable)

**Corrosion of Slab Reinforcement**  
(variable)

- 
- Together, caused the bulk of the critically low margins of safety
  - Existed from the time construction was complete – 40 years before the partial collapse

# Preliminary Pool Deck Evaluation

There were indications of severe distress in the pool deck at least three weeks before the collapse



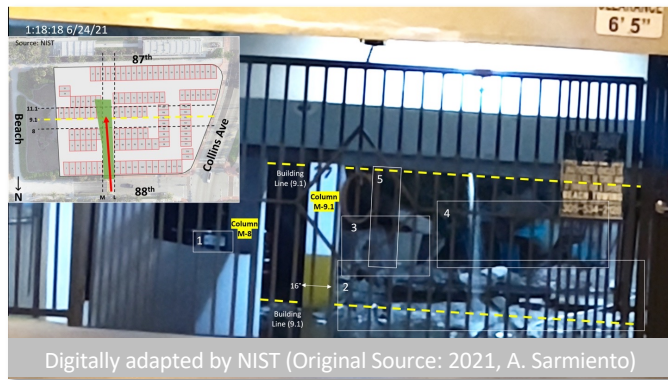
Source: NIST

PRELIMINARY ANALYSIS RESULTS

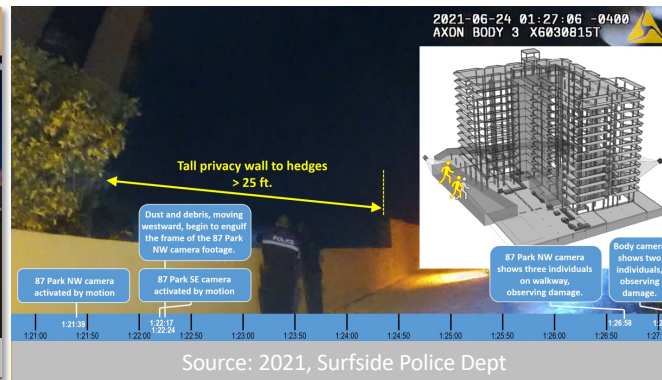
# Collapse Sequence

# Collapse Sequence

1. The pool deck collapsed between its southern extremity and its connection to the tower more than four minutes before the general collapse of the tower.



NIST's Analysis of CTS Parking Garage Ramp Video Footage



NIST's Analysis of Beach Access Walkway Video Footage



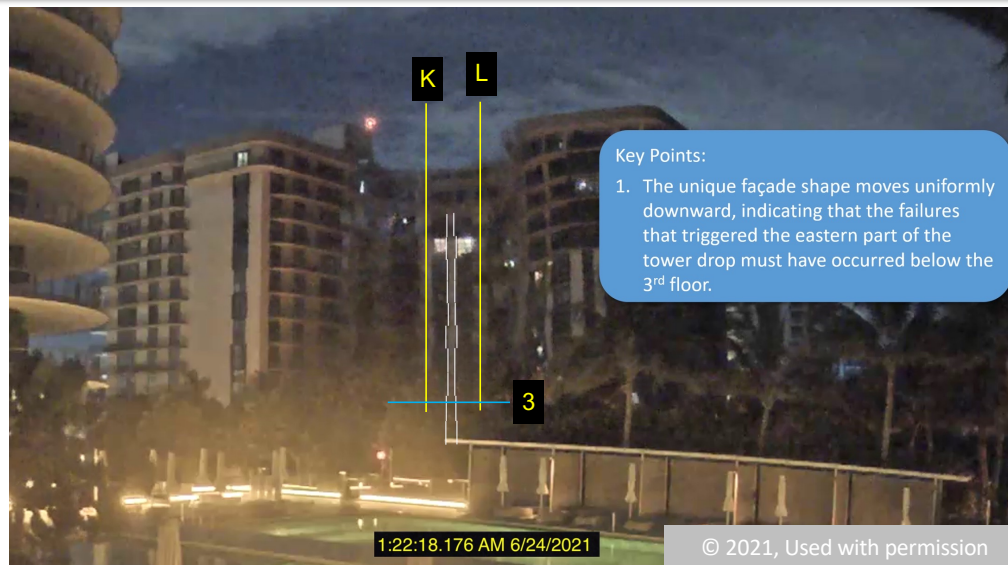
Eyewitness Accounts of Pool Deck Collapse

PRELIMINARY  
ANALYSIS RESULTS

# Collapse Sequence

2. In the tower collapse, Grid Line 9.1 started to drop a second, or a bit more, before 1:22:17 am, the time of the first frame of the South Face Video.

- The columns on Grid Line K and/or L dropped first.
- The initial column failures were low in the building below the 3<sup>rd</sup> floor.

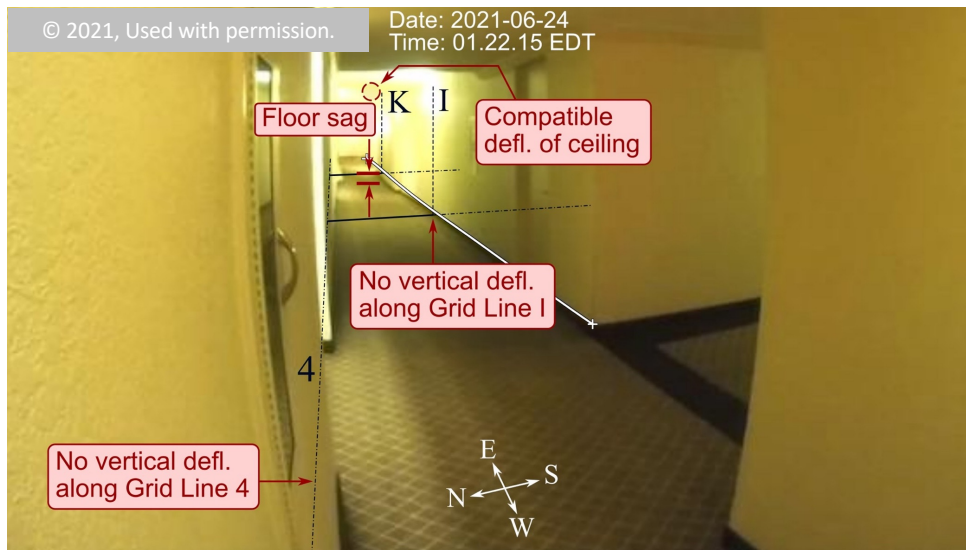


PRELIMINARY  
ANALYSIS RESULTS

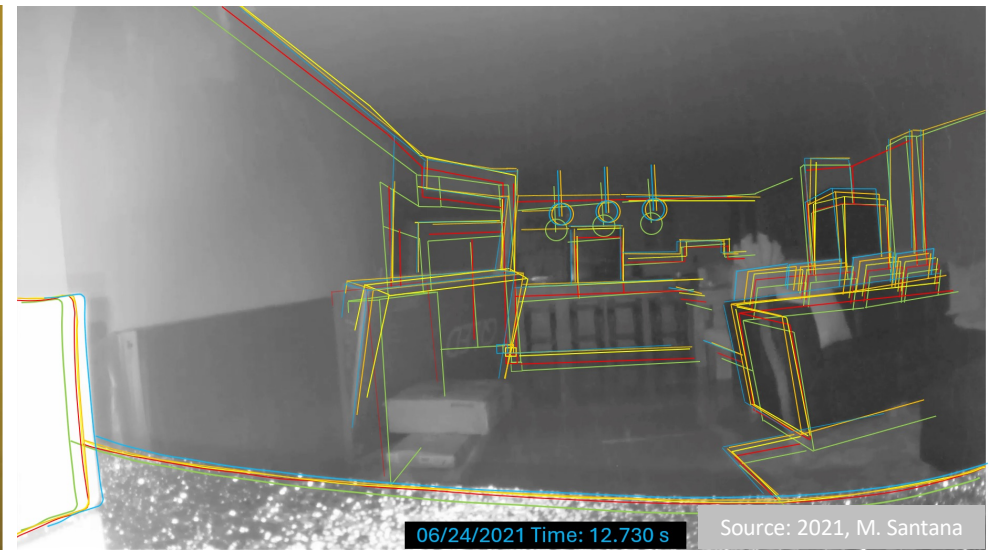
NIST's Analysis of South Face Video Footage

# Collapse Sequence

3. Videos show severe structural movements in the tower between Grid Lines K and M and Grid Lines 4 and 9.1 prior to the precipitous drop of the tower along Grid Line 9.1.



NIST's Analysis of Upper Story Corridor Video Footage

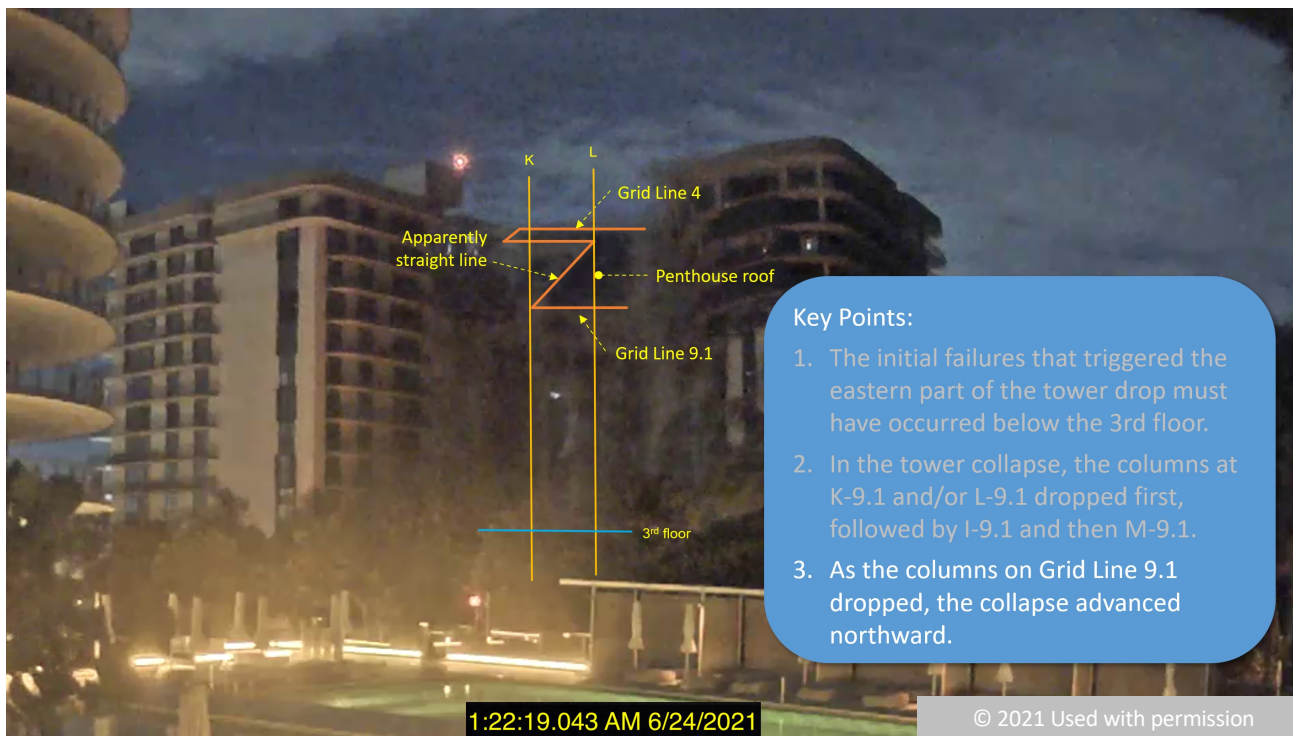


NIST's Analysis of 11 Stack Unit Video Footage

PRELIMINARY ANALYSIS RESULTS

# Collapse Sequence

4. As the columns on Grid Line 9.1 dropped, the collapse advanced northward.



## Key Points:

1. The initial failures that triggered the eastern part of the tower drop must have occurred below the 3rd floor.
2. In the tower collapse, the columns at K-9.1 and/or L-9.1 dropped first, followed by I-9.1 and then M-9.1.
3. As the columns on Grid Line 9.1 dropped, the collapse advanced northward.

PRELIMINARY ANALYSIS RESULTS

NIST's Analysis of South Face Video Footage

127

# Collapse Sequence

5. While there is strong evidence that the collapse initiated in the pool deck, we have not yet ruled out a failure initiation in some part of the tower that precipitated a collapse in the pool deck.
- There were indications of severe distress in the pool deck at least three weeks before the collapse.
  - There are also potential initiation points in the tower.



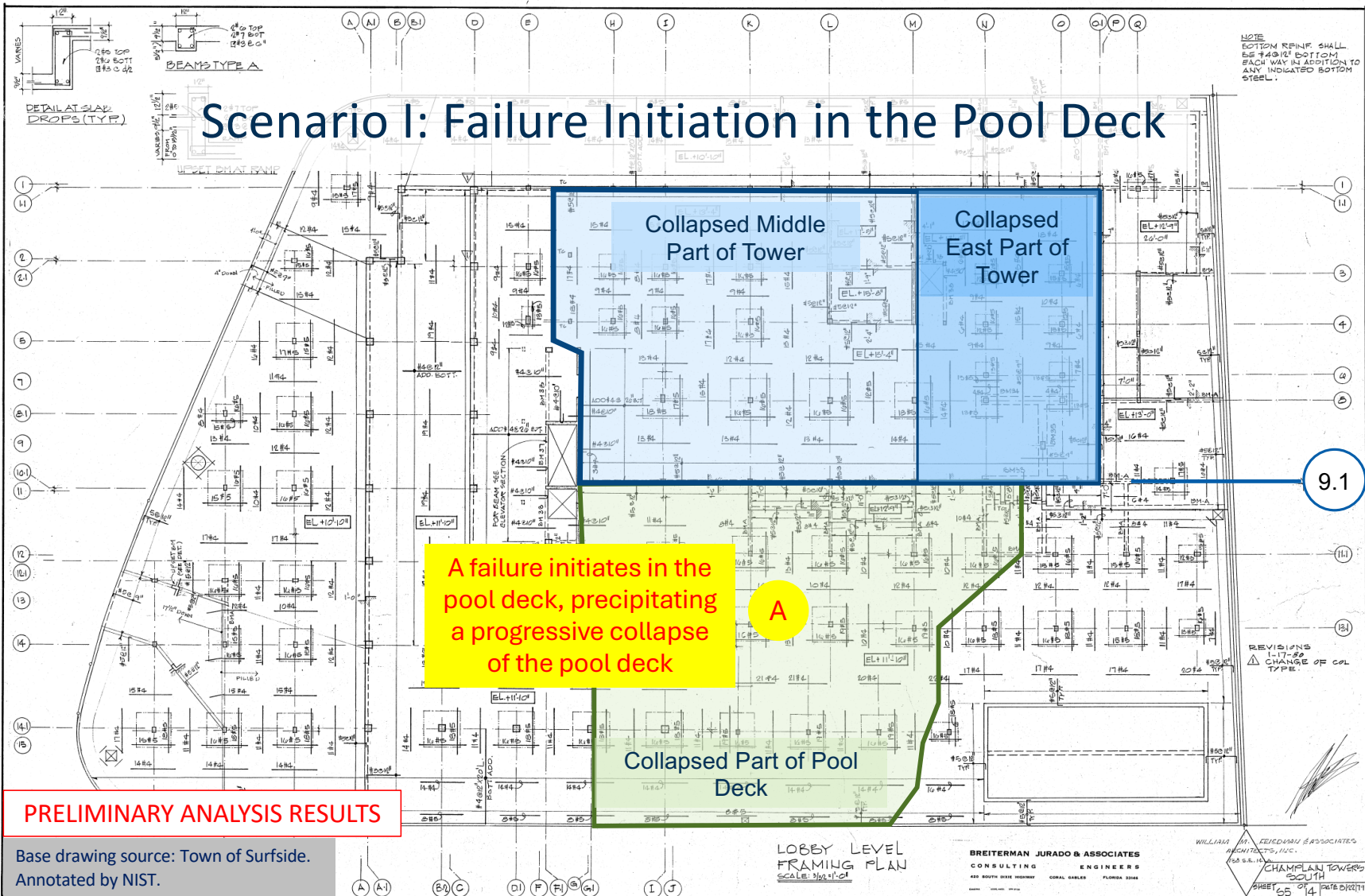
PRELIMINARY ANALYSIS  
RESULTS

Damage to Pool Deck Planters Three Weeks Before the Collapse Indicated Severe Structural Distress

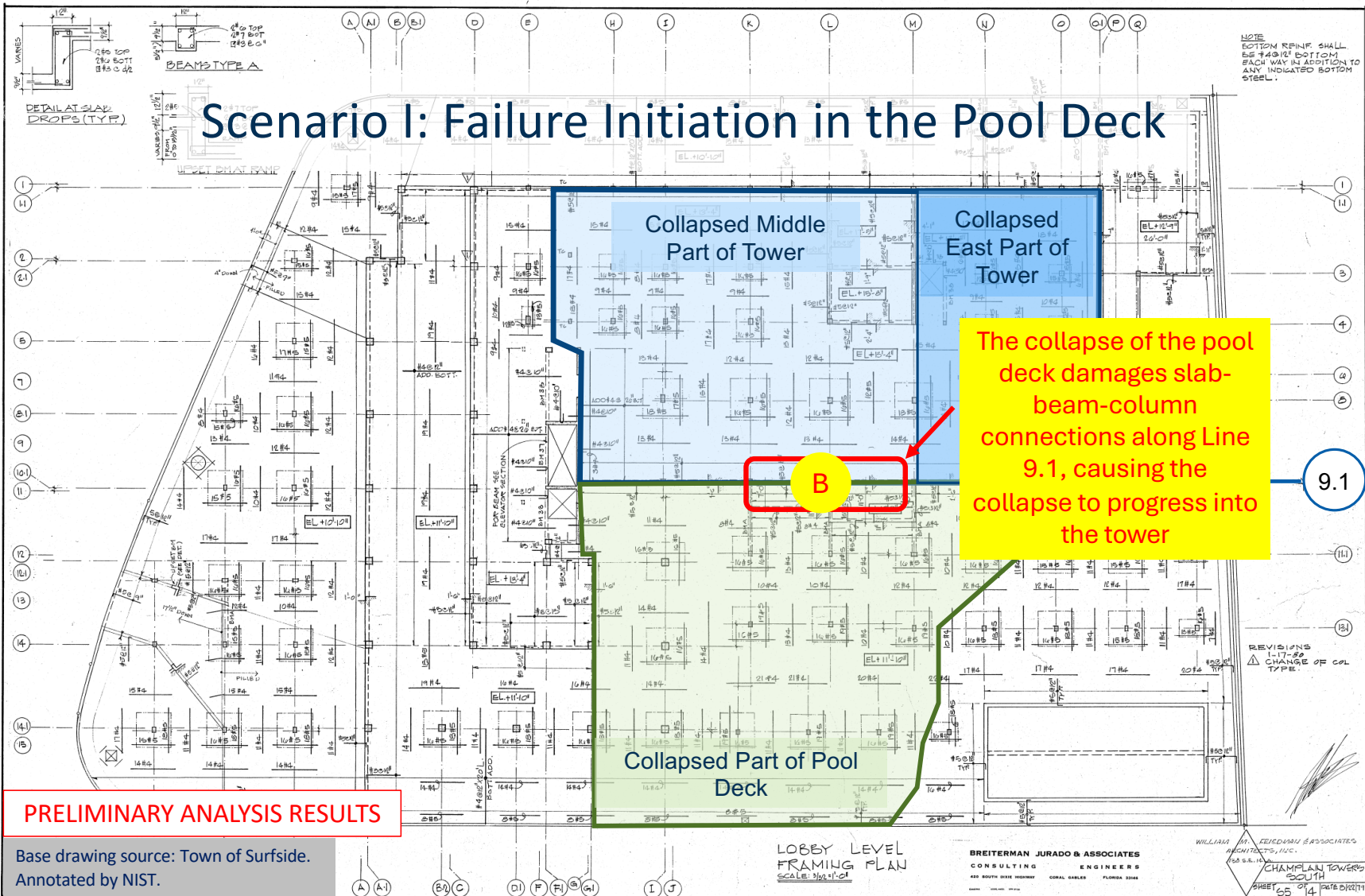


# Failure Initiation and Progression Scenarios

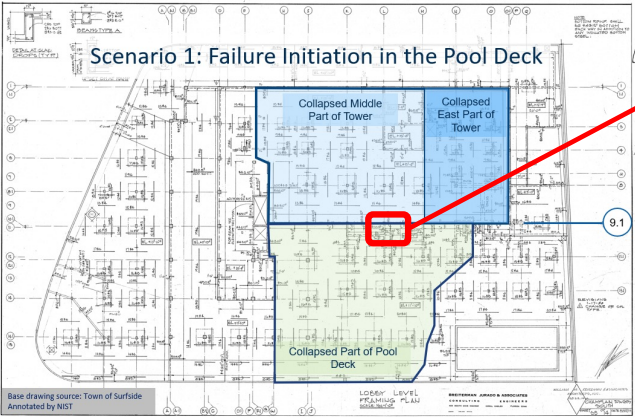
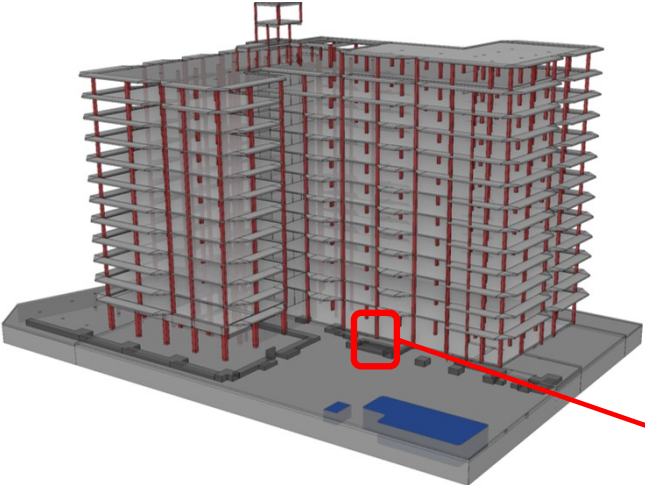
# Scenario I: Failure Initiation in the Pool Deck



# Scenario I: Failure Initiation in the Pool Deck

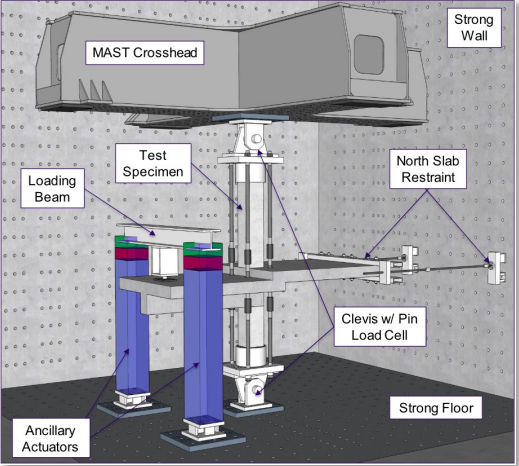


# Failure Initiation and Progression Scenarios

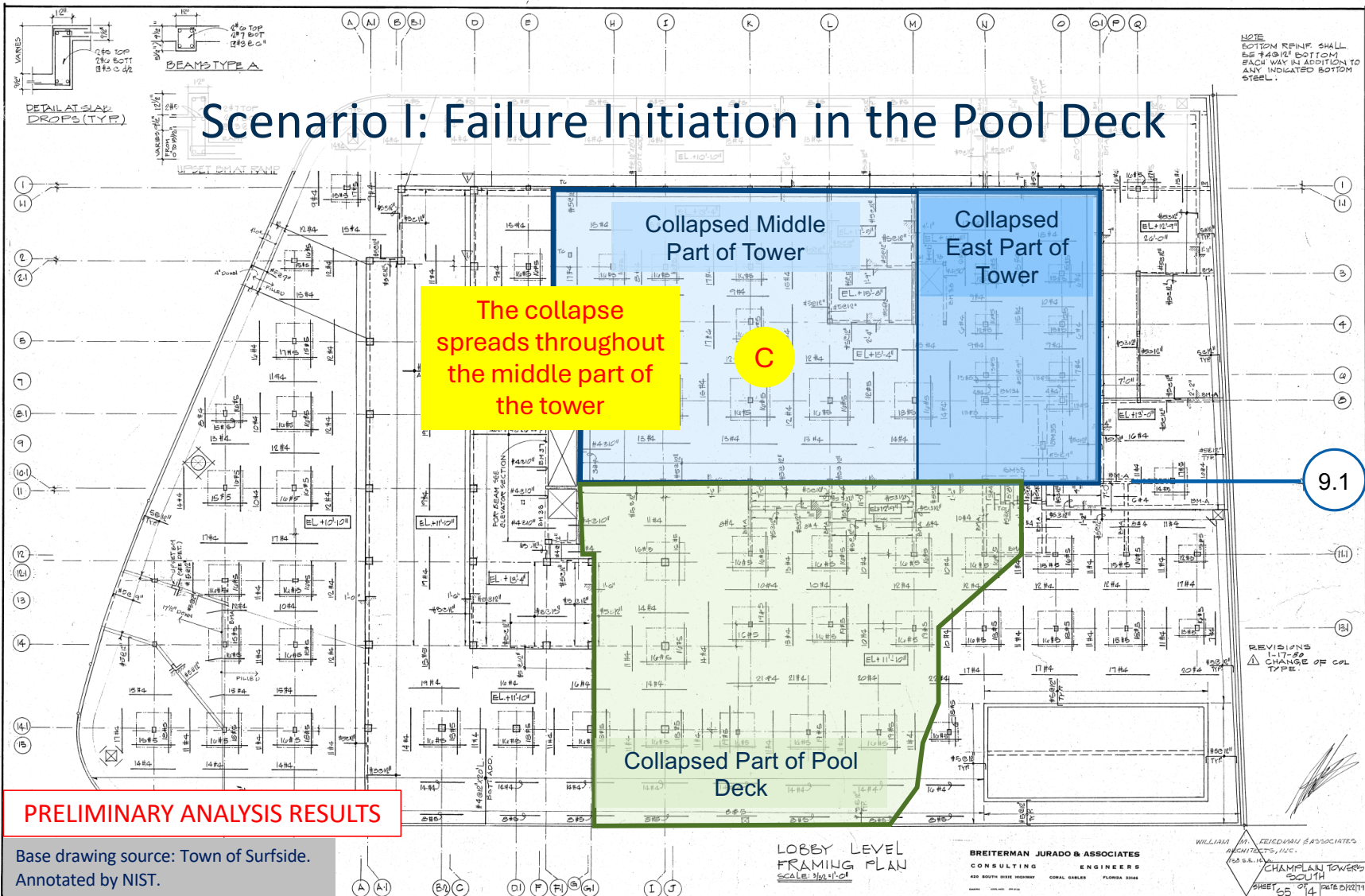


Source: NIST except where noted

**PRELIMINARY ANALYSIS RESULTS**



# Scenario I: Failure Initiation in the Pool Deck



## PRELIMINARY ANALYSIS RESULTS

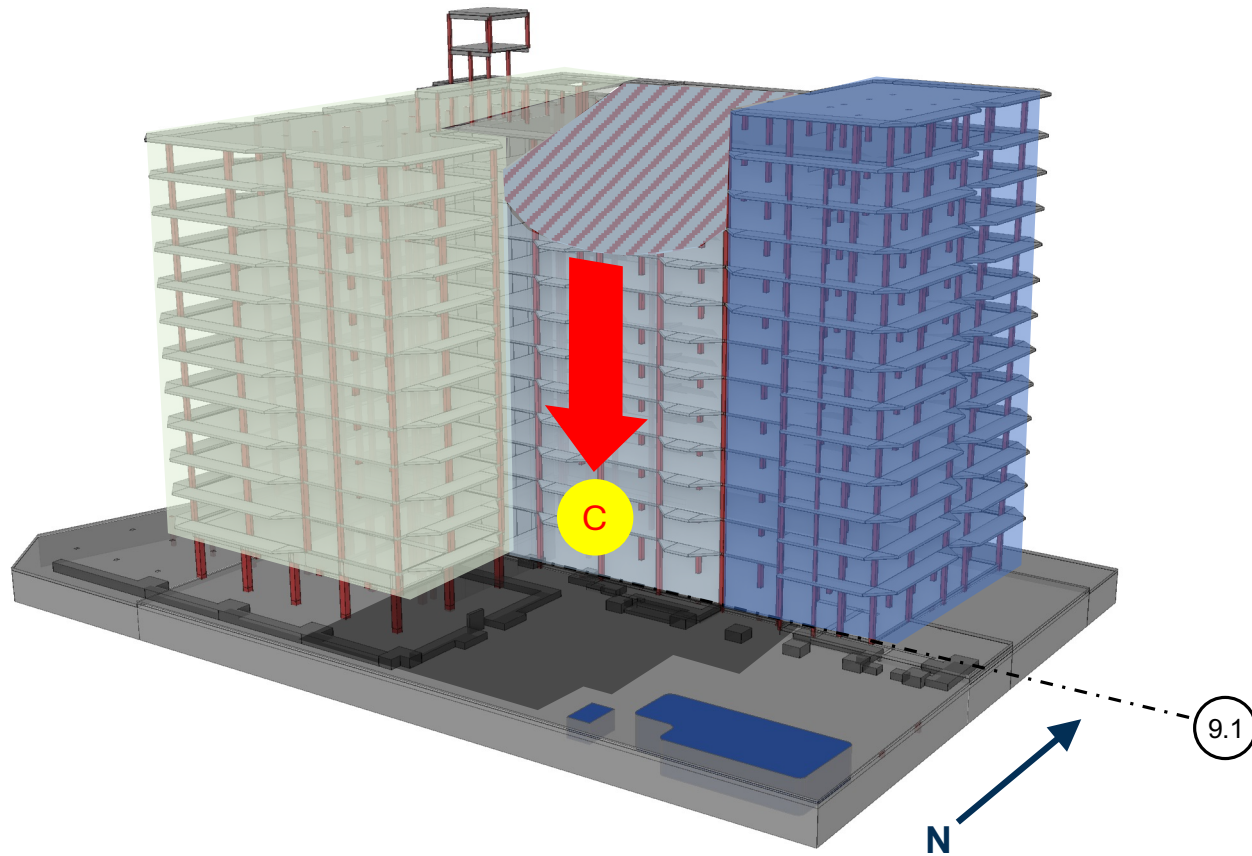
Base drawing source: Town of Surfside.  
Annotated by NIST.

LOBBY LEVEL  
FRAMING PLAN  
SCALE: 3/8"=1'-0"

BREITERMAN JURADO & ASSOCIATES  
CONSULTING ENGINEERS  
430 SOUTH WISE HIGHWAY CORAL GABLES FLORIDA 33134

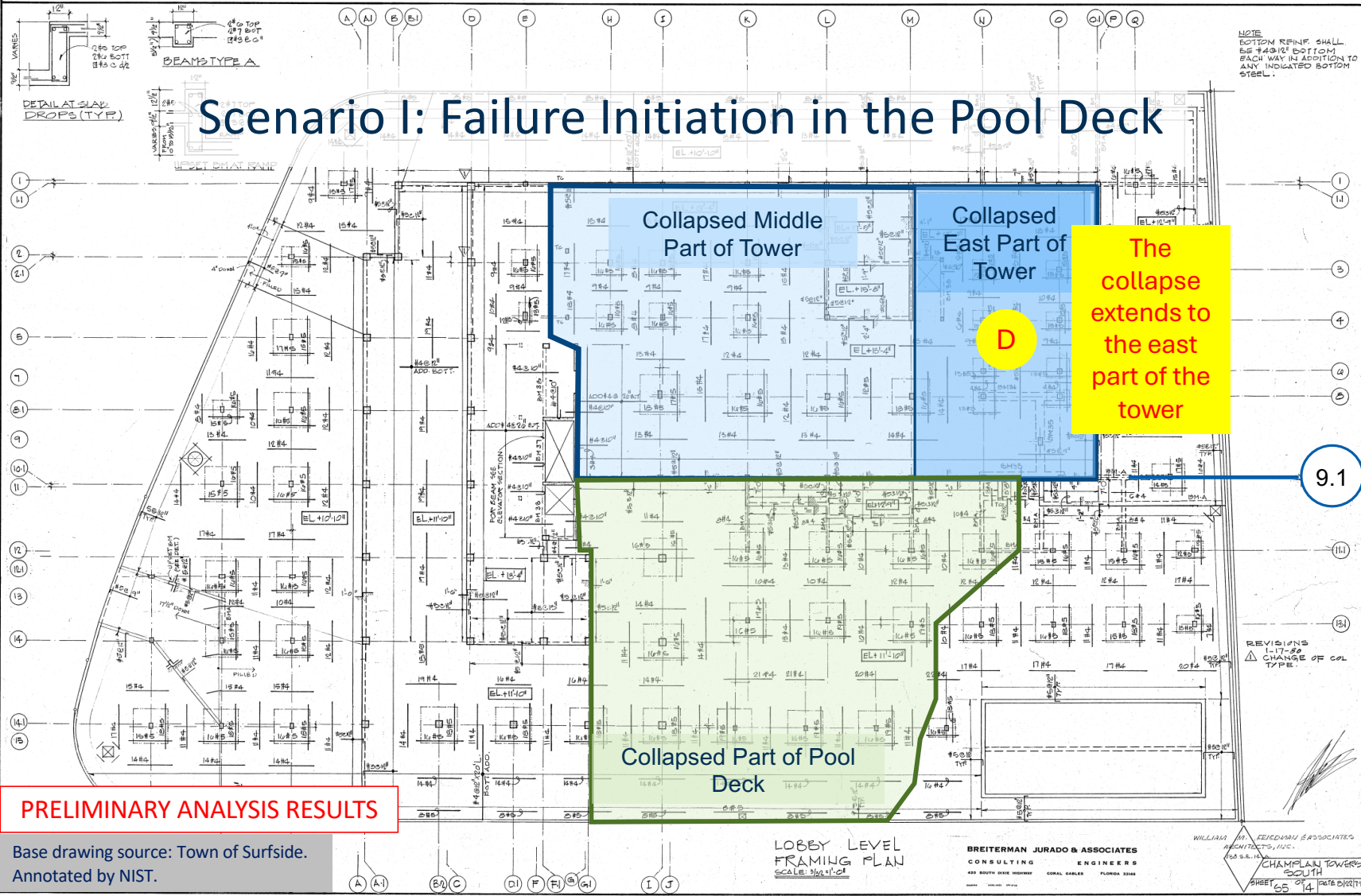
WILLIAM H. FREEDMAN & ASSOCIATES  
ARCHITECTS, INC.  
508 S.W. 11th  
CHAMPLAIN TOWERS  
SOUTH  
SHEET 25 OF 44

# Failure Initiation and Progression Scenarios



Source: NIST

# Scenario I: Failure Initiation in the Pool Deck



**PRELIMINARY ANALYSIS RESULTS**

Base drawing source: Town of Surfside.  
Annotated by NIST.

LOBBY LEVEL  
FRAMING PLAN  
SCALE: 3/8" = 1'-0"

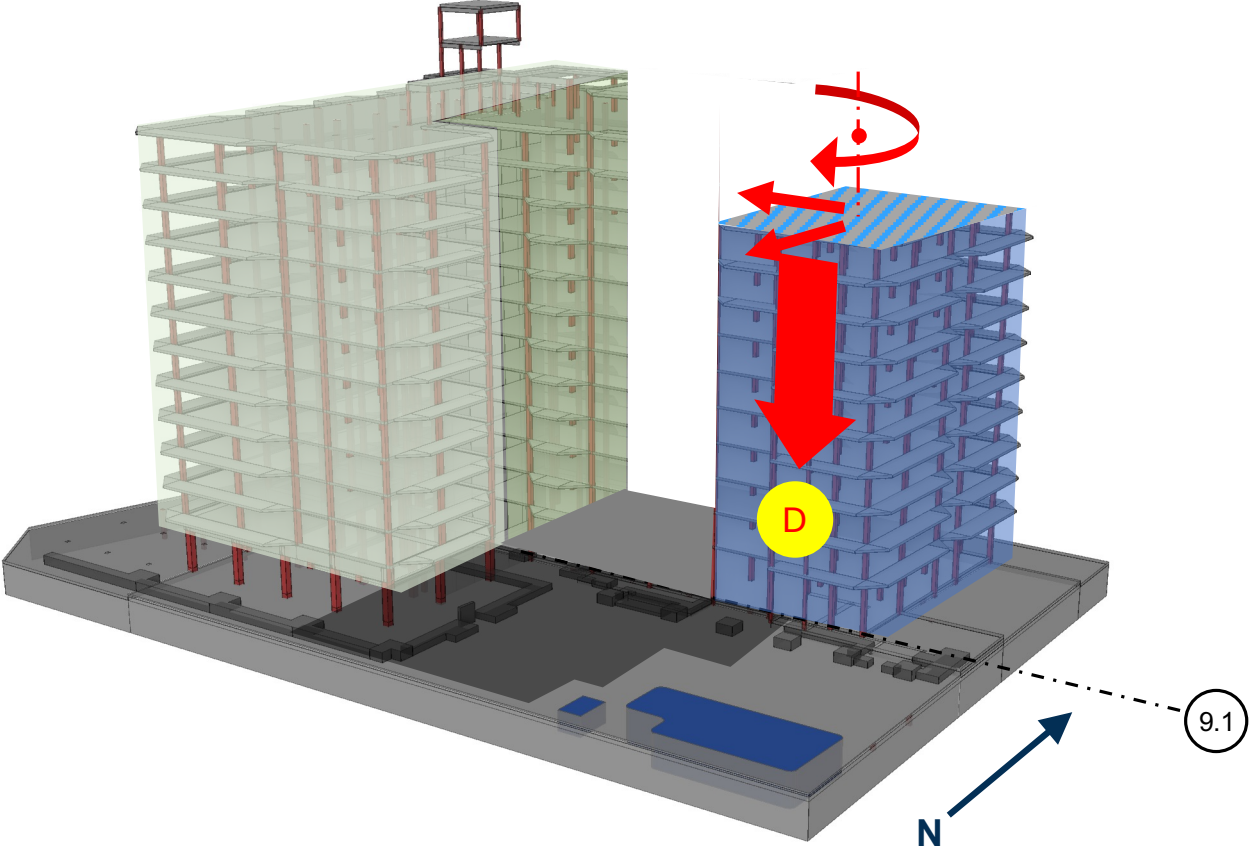
BREITERMAN JURADO & ASSOCIATES  
CONSULTING ENGINEERS  
430 SOUTH WISE HIGHWAY CORAL GABLES FLORIDA 33134

WILLIAM H. FEUERMAN & ASSOCIATES  
ARCHITECTS, INC.  
508 S.W. 11th  
CHAMPLAIN TOWERS  
SOUTH  
SHEET 25 OF 24 (DATE 02/27/11)

NOTE:  
BOTTOM REINFC SHALL  
BE #4@12" BOTTOM  
EACH WAY IN ADDITION TO  
ANY INDICATED BOTTOM  
STEEL.

REVISIONS  
1-17-10  
CHANGE OF COL  
TYPE.

# Failure Initiation and Progression Scenarios



Source: NIST



# Scenario II: Failure Initiation in the Tower

A failure initiates in the tower but does not immediately cause a collapse of the tower

A

Collapsed Middle Part of Tower

Collapsed East Part of Tower

Collapsed Part of Pool Deck

NOTE  
BOTTOM REINF SHALL BE #4@2' BOTTOM EACH WAY IN ADDITION TO ANY INDICATED BOTTOM STEEL.

9.1

PRELIMINARY ANALYSIS RESULTS

Base drawing source: Town of Surfside.  
Annotated by NIST.

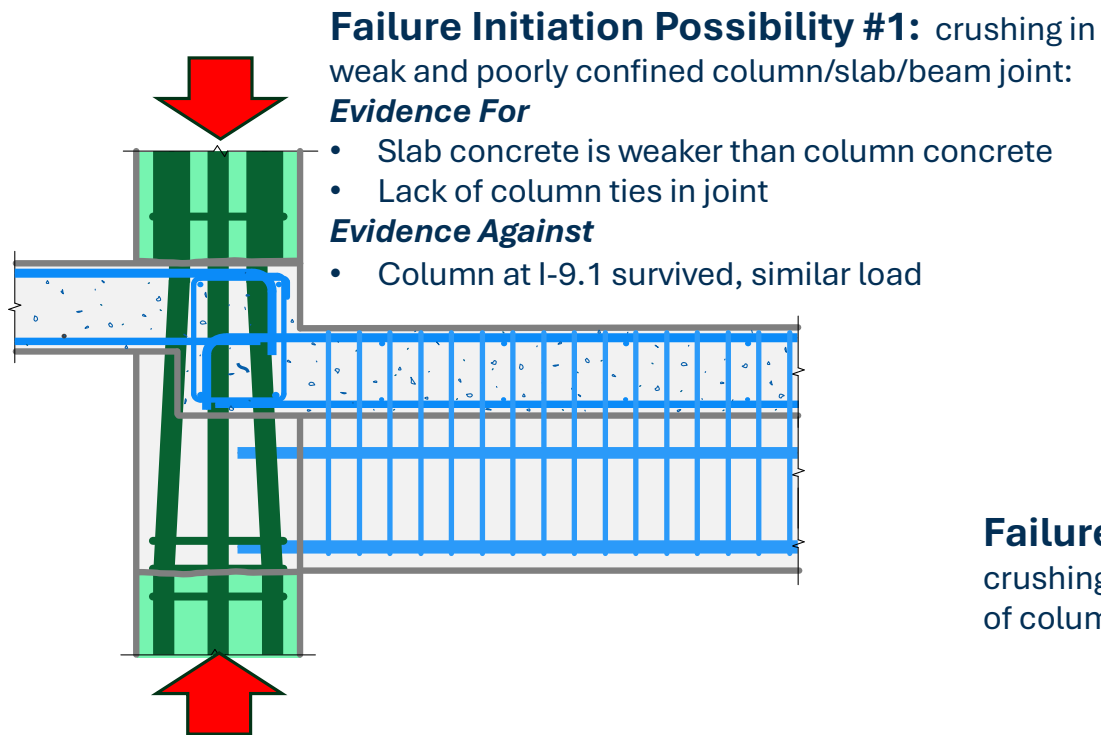
LOBBY LEVEL  
FRAMING PLAN  
SCALE: 3/8"=1'-0"

BREITERMAN JURADO & ASSOCIATES  
CONSULTING ENGINEERS  
430 SOUTH WISE HIGHWAY CORAL GABLES FLORIDA 33134

WILLIAM H. FREEDMAN & ASSOCIATES ARCHITECTS, INC.  
508 S.W. 11th  
CHAMPLAIN TOWERS SOUTH  
SHEET 25 OF 44 DATE 02/27/11

## Description of Three Scenario II Failure Initiation Possibilities

All three possibilities lead to shortening of column and redistribution of load elsewhere.



### Failure Initiation

**Possibility #2:** partial failure at improper lap splice:

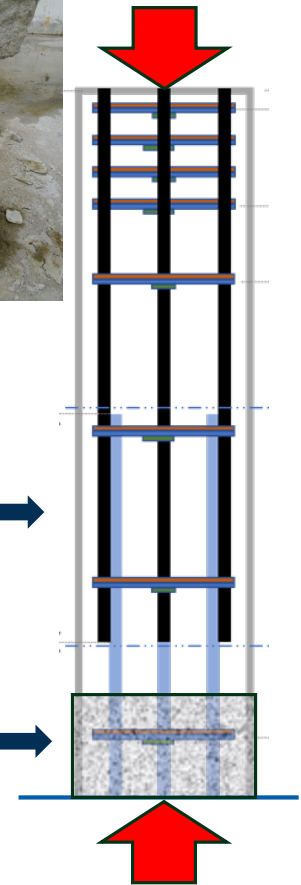
**Evidence For:** measurements

**Evidence Against:** survival of columns with short splices that permitted such measurements

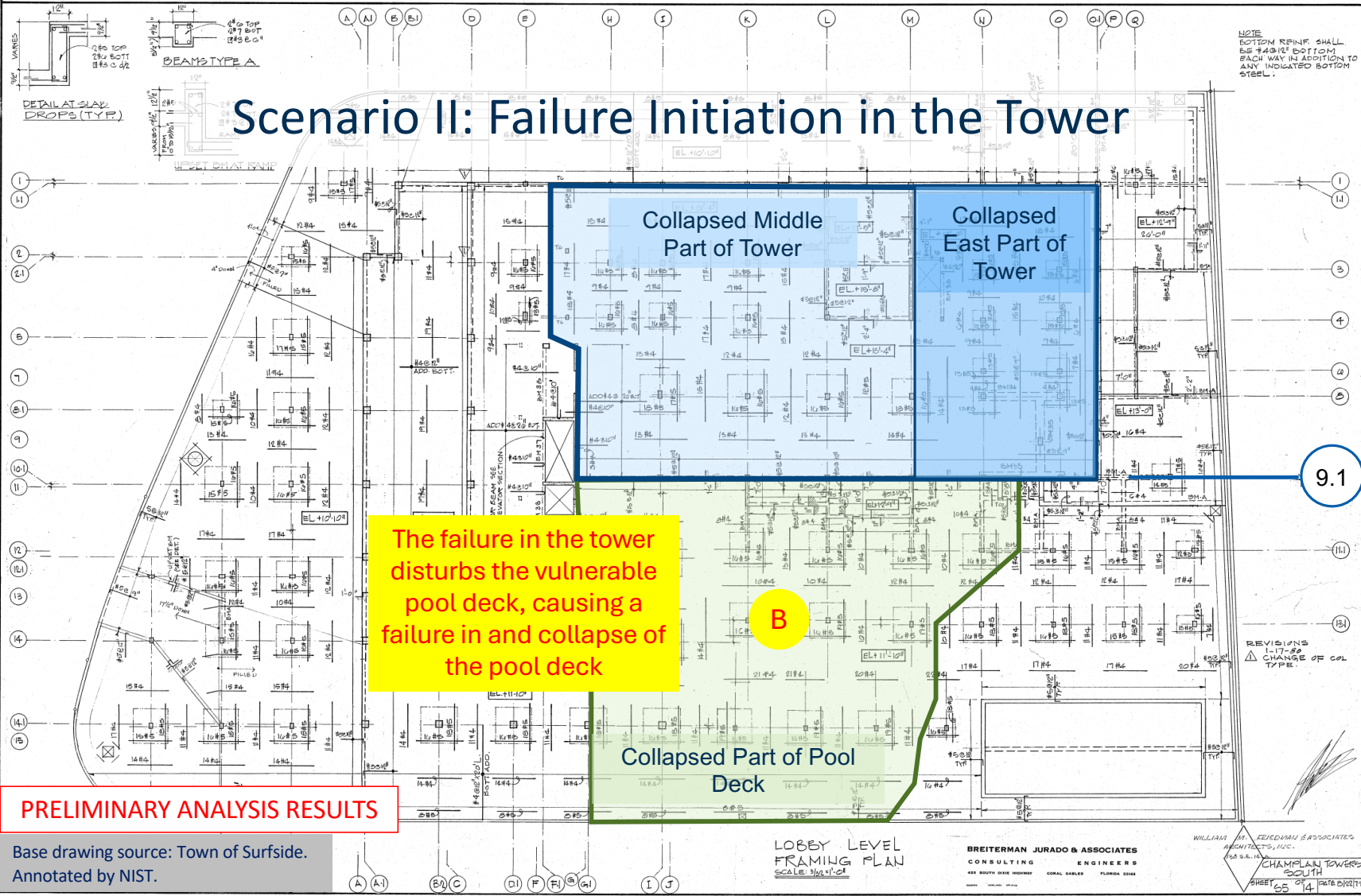


### Failure Initiation Possibility #3:

crushing in deteriorated concrete at bottom of column in basement



# Scenario II: Failure Initiation in the Tower



The failure in the tower disturbs the vulnerable pool deck, causing a failure in and collapse of the pool deck

**PRELIMINARY ANALYSIS RESULTS**

Base drawing source: Town of Surfside.  
Annotated by NIST.

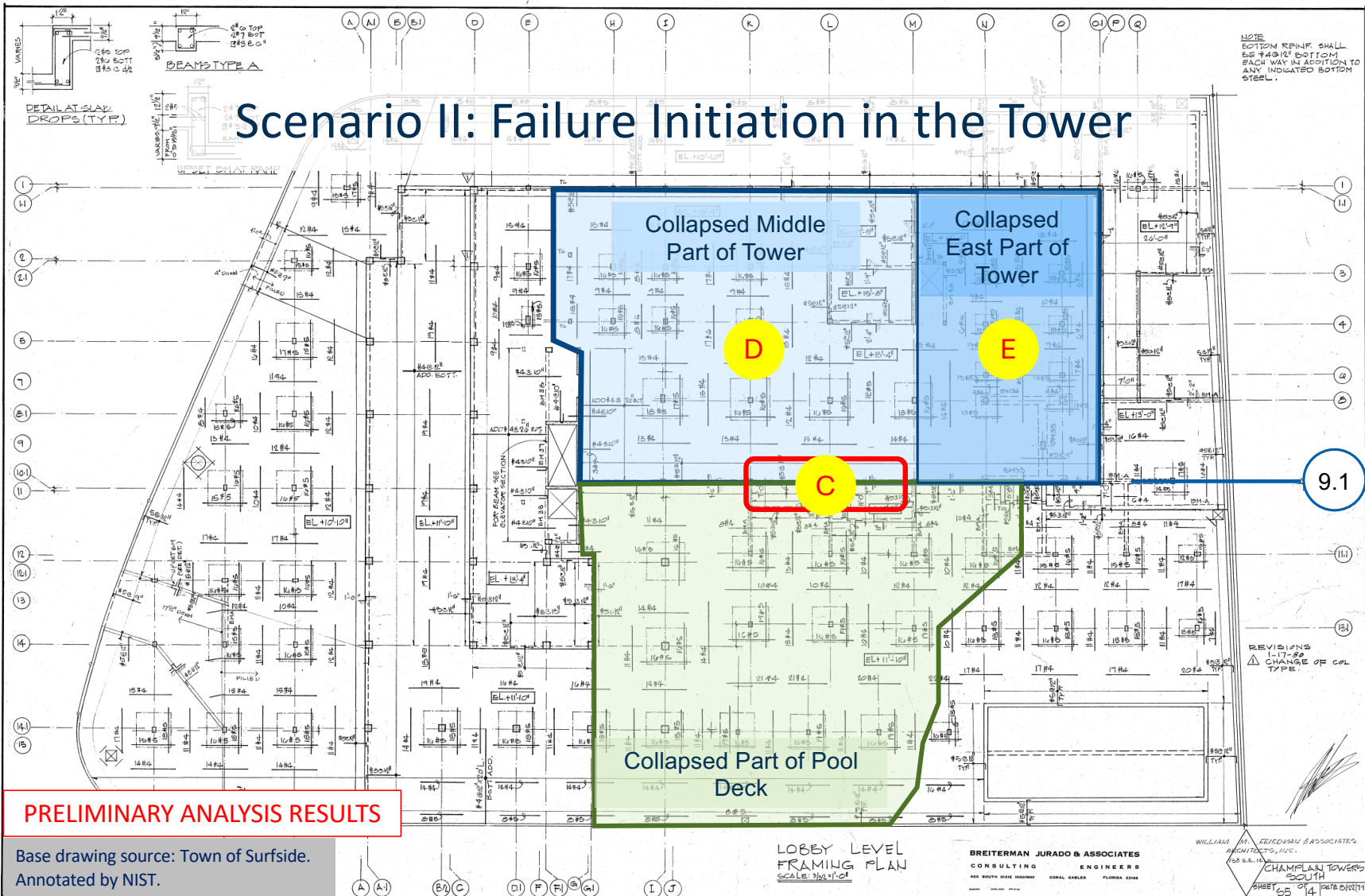
LOBBY LEVEL  
FRAMING PLAN  
SCALE: 3/8" = 1'-0"

BREITERMAN JURADO & ASSOCIATES  
CONSULTING ENGINEERS  
480 SOUTH MIKE HODSON CORAL GABLES FLORIDA 33134

WILLIAM H. FREEDMAN & ASSOCIATES ARCHITECTS, INC.  
508 S.W. 11th  
CHAMPLAIN TOWERS SOUTH  
SHEET 25 OF 44 DATE 02/27/11

9.1

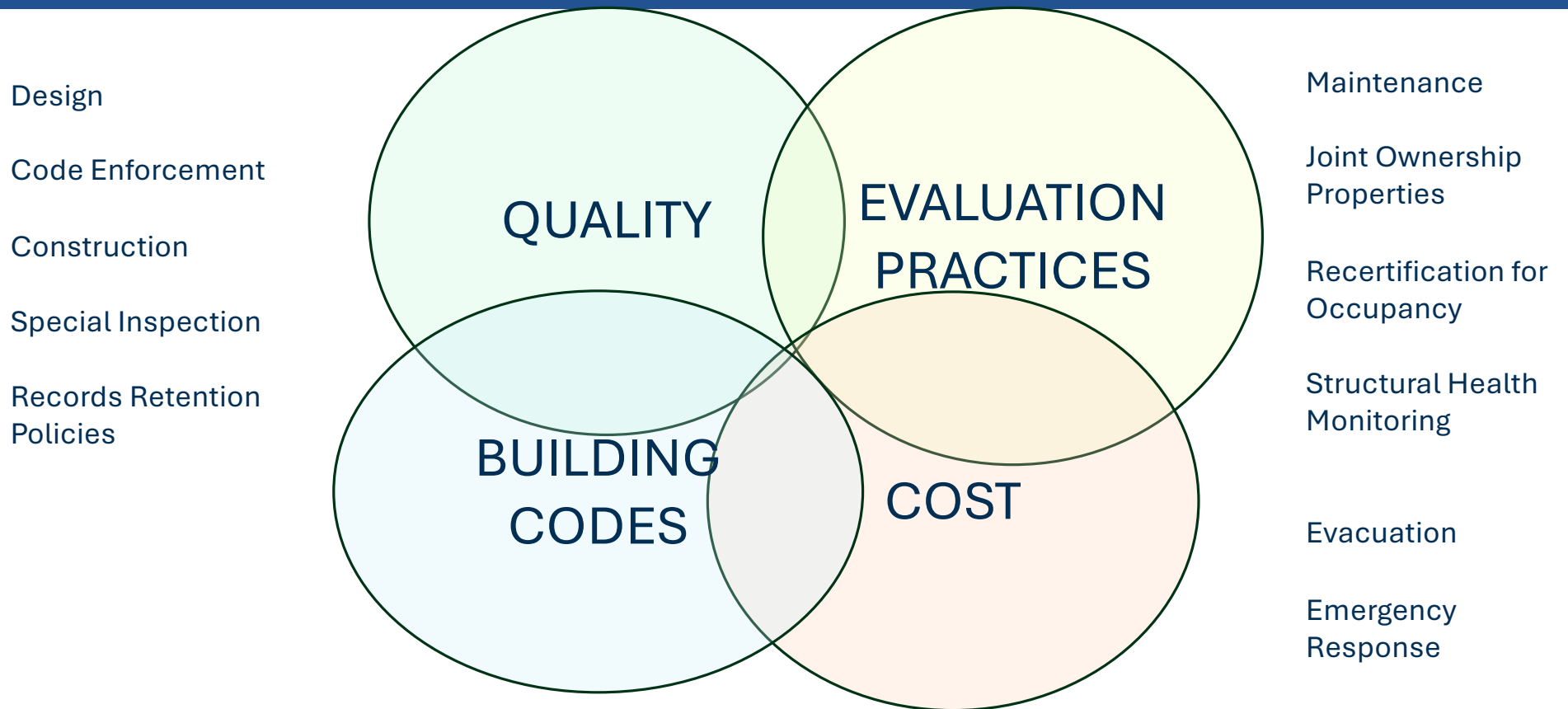
# Scenario II: Failure Initiation in the Tower



06

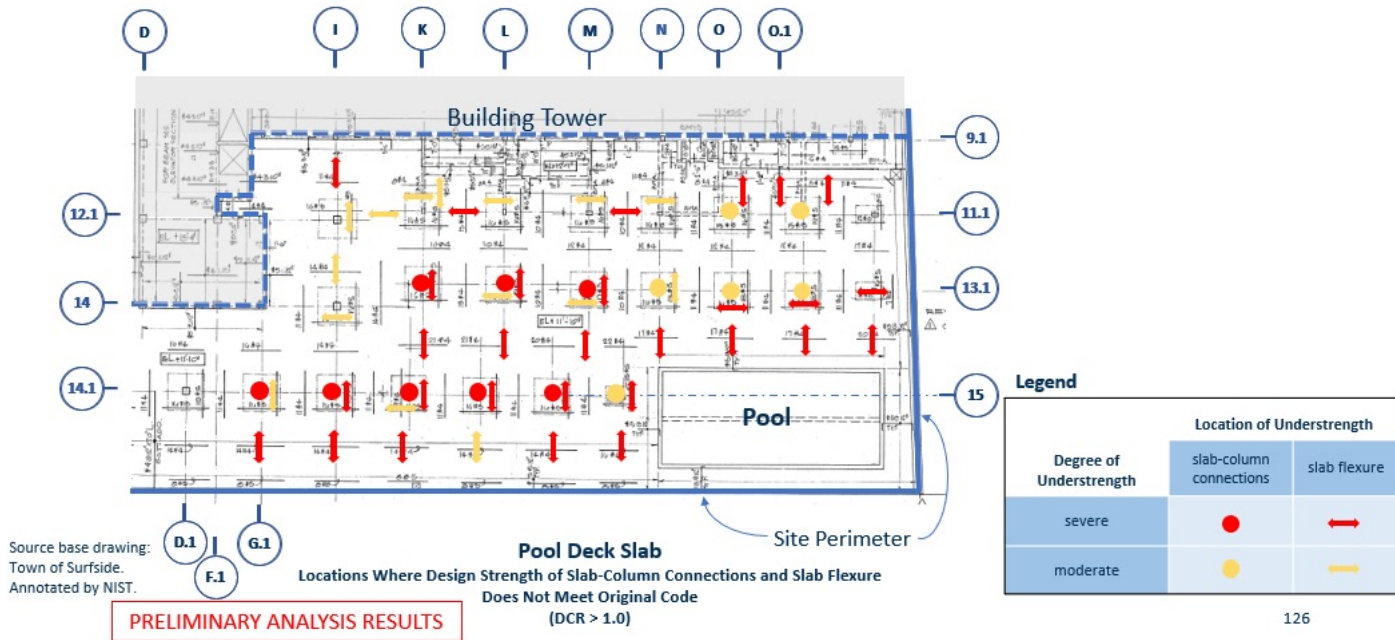
Codes and Standards of  
Practice, Potential  
Recommendation Topics

## Potential Topics for Recommendations



THERE WILL BE A LOT TO TALK ABOUT!

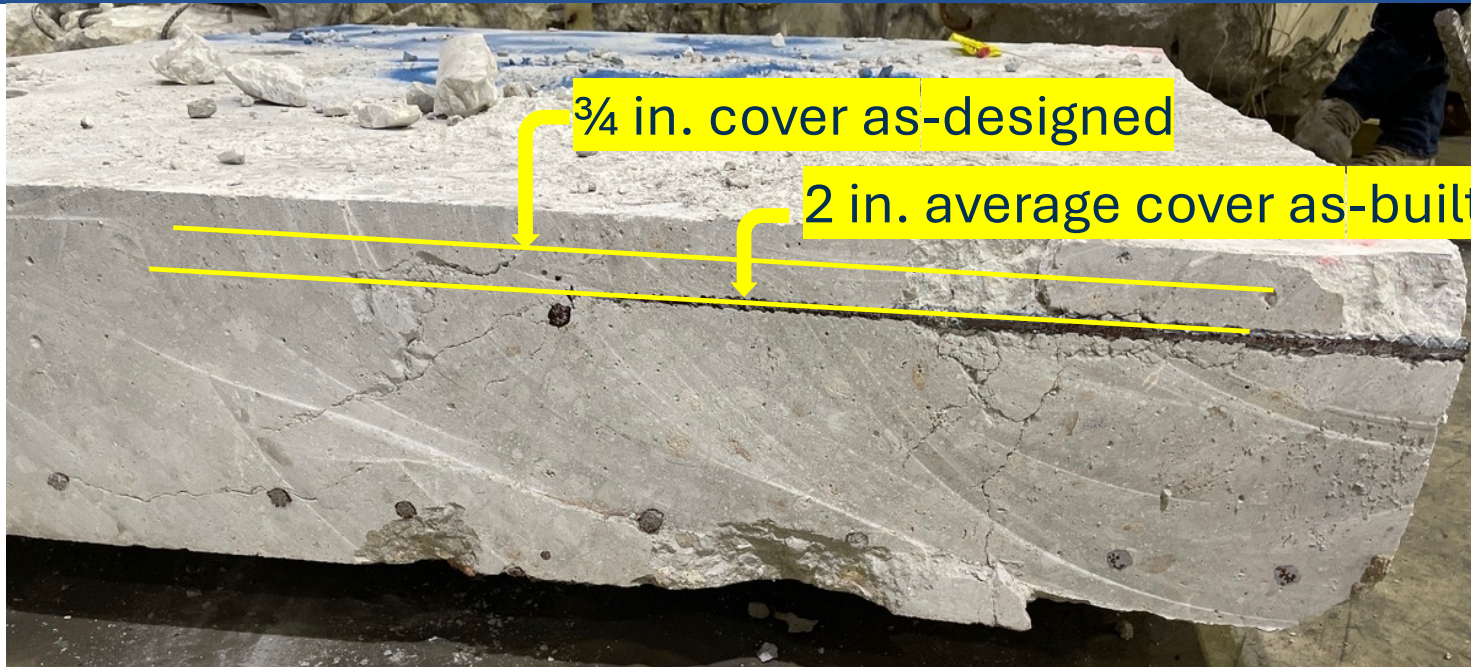
# Quality Control/Assurance in Design



Building codes change over time, but our review showed significant deficiencies under both the codes in effect at original design and those of today.

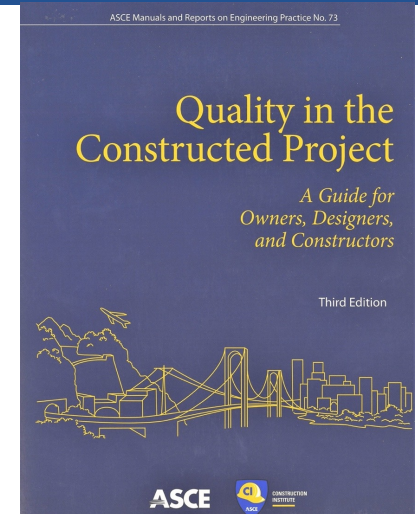
- Quality control starts with the design engineer and the engineer of record.
- Quality assurance is external, and can be provided by peer reviews, governmental agencies, or other.

# Quality Control/Assurance in Construction



Source: NIST

Quality control starts with the installer and proceeds through foreman and superintendent. Quality assurance is external, and it is often provided by code-mandated special inspectors.



Source: ASCE



# Design Codes

- There have been many changes in structural design codes over the years since CTS was built.
  - Minimum flexural reinforcement over columns in flat slabs
  - Reinforcement for structural integrity
  - Concrete properties and cover for high chloride exposure
  - Detailing of beam-column joints
  - Higher design wind speeds for hurricanes
- The effect of those changes is being considered, and will deserve more discussion, as well as the need for additional changes

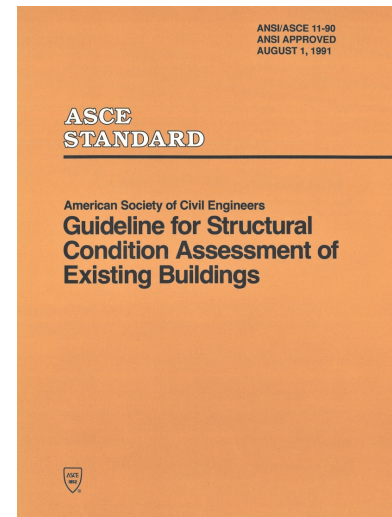


Source: ACI

Source: ASCE

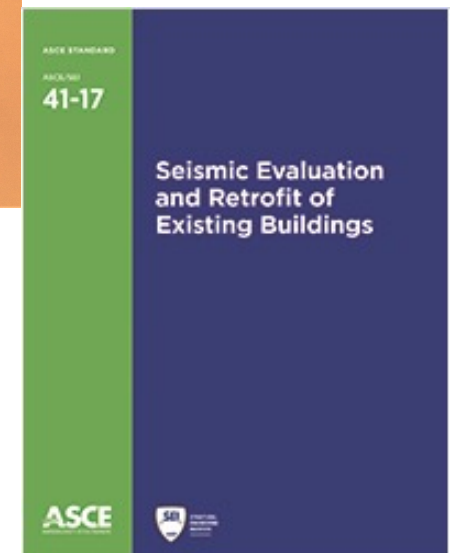
# Evaluation Requirements and Guides

- Miami-Dade's ordinance requiring recertification once a building reached 40 years of age began in 1975
- State of Florida implement a new law following the collapse of CTS beginning the recertification at age 30 years
- Both are focused on degradation
- *Is this enough?*



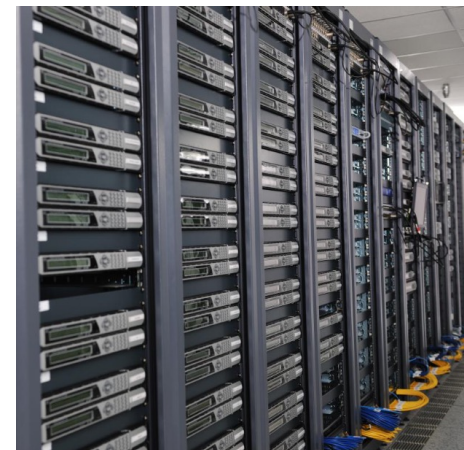
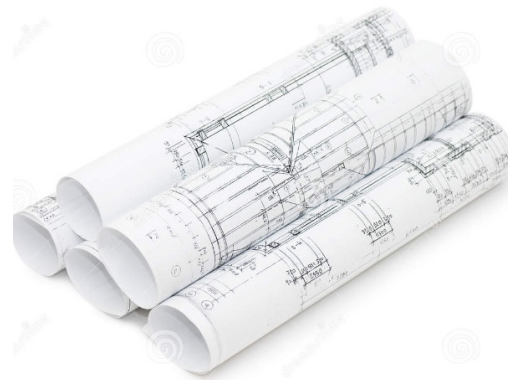
Source: ASCE

*Manual for Bridge Evaluation*  
(AASHTO)



# Records Retention

- One key aspect of controlling the cost of evaluation is the retention of records from the original design and construction, any peer review, and prior evaluations.
- As we transition from paper to digital records, this process needs to be carefully vetted.



07

Closing Comments,  
Impact of the Investigation

# NIST's Investigation of the Champlain Towers South Partial Collapse

## NIST CTS Information

<https://www.nist.gov/champlain>



## Public Meeting Videos

<https://www.nist.gov/disaster-failure-studies/champlain-towers-south-collapse-ncst-investigation/public-meeting-videos>



## NIST DFS Portal

<https://www.nist.gov/disaster-failure-studies/data-submission-portal>



**STRUCTURES CONGRESS 2025 | [structurescongress.org](https://structurescongress.org)**